

Appendix F
Transportation Study



HEXAGON TRANSPORTATION CONSULTANTS, INC.

4146 Mitzi Drive Residential Development

Transportation Analysis

Prepared for:

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November 29, 2018



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Executive Summary

This report presents the results of the transportation analysis (TA) for the proposed residential homes at 4146 Mitzi Drive in the City of San Jose. The project proposes to construct a 40-unit apartment building and to convert a single-family residence into a multi-family building with 6 units. Thus, the project totals 46 dwelling units. The project would have a single driveway off Mitzi Drive leading to an underground parking garage serving all 46 units.

This study was conducted for the purpose of identifying the potential transportation impacts related to the proposed development. The transportation impacts of the project were evaluated following the standards and methodologies established in the City of San Jose's *Transportation Analysis Handbook*. Based on the City of San Jose's Transportation Analysis Policy and *Transportation Analysis Handbook*, the TA report for the project includes a CEQA transportation analysis and a local transportation analysis (LTA).

CEQA Transportation Impacts

Project Vehicle Miles Traveled (VMT) Analysis

The VMT generated by the project (8.52 VMT per capita) would be less than the threshold of 10.12 VMT per capita. Therefore, the project would not result in a significant transportation impact on VMT. Therefore, no mitigation measures are required to reduce VMT.

CEQA Cumulative Impacts

The project is consistent with the General Plan goals and policies for the following reasons:

- The project site is near bus stops on Saratoga Avenue and Williams Road and bicycle lanes on Williams Road.
- The project would increase the residential density in the project area.
- The project would provide bicycle parking.

Therefore, the project would be considered as part of the cumulative solution to meet the General Plan's long-range transportation goals and would result in a less-than-significant cumulative impact.

Local Transportation Effects

Project Trip Generation

Vehicle trips that would be generated by the project were estimated using the trip generation rates published in the Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 10th Edition (2017), for Single-Family Detached Housing (Land Use Code 210) and Mid-Rise Multifamily Housing (Land Use Code 221).

The project is estimated to generate 209 daily vehicle trips, with 14 trips occurring during the AM peak hour and 17 trips occurring during the PM peak hour. Using the inbound/outbound splits contained in ITE's *Trip Generation Manual*, the project would produce 4 inbound and 10 outbound trips during the AM peak hour, and 10 inbound and 7 outbound trips during the PM peak hour.

Intersection Traffic Operations

The results of the analysis show that the signalized intersection of Saratoga Avenue and Williams Road intersection currently operates at an acceptable level of service (LOS D or better) during the AM and PM peak hour, which meets the San Jose standard (see Table ES-1).

The results of the analysis show that the left turn from Piper Drive to Saratoga Avenue operates at LOS F during the AM peak hour. However, the City of San Jose does not have a level of service standard for unsignalized intersections. The traffic count showed that there were only 17 vehicles making the left-turn movement during the AM peak hour. Field observations showed that there was a maximum queue of 2 vehicles making the left turn from Piper Drive onto Saratoga Avenue during the AM peak hour.

Other Transportation Issues

The proposed site plan shows adequate site access and on-site circulation. The project would enhance pedestrian circulation by providing sidewalks along its frontage, which are lacking today. The existing transit services and bicycle facilities are sufficient to serve the project.

Hexagon has the following recommendations resulting from the parking, site access, and circulation analysis.

- The project site plan should be revised to meet the City's minimum requirement of 26 feet for a two-way driveway.
- Since the stalls on the dead-end aisle are likely to be used by guests, turnaround space should be provided. This could be accomplished by removing one of the parking stalls.
- The bicycle parking should be moved to the ground floor.
- Landscaping near the driveway should be kept below three feet to maintain sight distance.

**Table ES 1
Intersection Level of Service Summary**

| # | Intersection | Peak Hour | Count Date | Existing Conditions | | Background Conditions | | | | | | Cumulative Conditions | |
|---|--|-----------|------------|--------------------------|----------|--------------------------|----------|--------------------------|----------|-------------------------------|-----------------------|--------------------------|----------|
| | | | | Delay (sec) ¹ | LOS | No Project | | with Project | | | | No Project | |
| | | | | | | Delay (sec) ¹ | LOS | Delay (sec) ¹ | LOS | Incr. in Critical Delay (sec) | Incr. in Critical V/C | Delay (sec) ¹ | LOS |
| 1 | Ranchero Way and Williams Road (unsignalized) | AM | 11/6/18 | 23.6 | C | 23.6 | C | 23.7 | C | 0.1 | 0.001 | 23.6 | C |
| | | PM | 11/6/18 | 21.6 | C | 21.6 | C | 21.7 | C | 0.0 | 0.002 | 21.6 | C |
| 2 | Saratoga Avenue and Piper Drive (unsignalized) | AM | 11/6/18 | >90 | F | >90 | F | >90 | F | 0.2 | 0.038 | >90 | F |
| | | PM | 11/6/18 | 41.3 | E | 41.3 | E | 41.4 | E | 0.1 | 0.024 | 43.3 | E |
| 3 | Saratoga Avenue and Mitzi Drive (unsignalized) | AM | 11/6/18 | 15.1 | B | 15.1 | B | 15.1 | B | 0.0 | 0.000 | 15.1 | B |
| | | PM | 11/6/18 | 11.7 | B | 11.7 | B | 11.7 | B | 0.0 | 0.000 | 11.8 | B |
| 4 | Piper Drive and Mitzi Drive (unsignalized) | AM | 11/6/18 | 8.7 | A | 8.7 | A | 8.8 | A | 0.4 | 0.003 | 8.7 | A |
| | | PM | 11/6/18 | 8.6 | A | 8.6 | A | 8.7 | A | 0.2 | 0.000 | 8.6 | A |
| 5 | Ranchero Way and Mitzi Drive (unsignalized) | AM | 11/6/18 | 9.2 | A | 9.2 | A | 9.2 | A | 0.0 | 0.000 | 9.2 | A |
| | | PM | 11/6/18 | 9.2 | A | 9.2 | A | 9.3 | A | 0.0 | 0.000 | 9.2 | A |
| 6 | Saratoga Avenue and Williams Road | AM | 11/6/18 | 36.9 | D | 37.0 | D | 37.0 | D | 0.0 | 0.001 | 36.9 | D |
| | | PM | 11/6/18 | 41.1 | D | 40.5 | D | 40.5 | D | 0.1 | 0.002 | 40.4 | D |

Note:

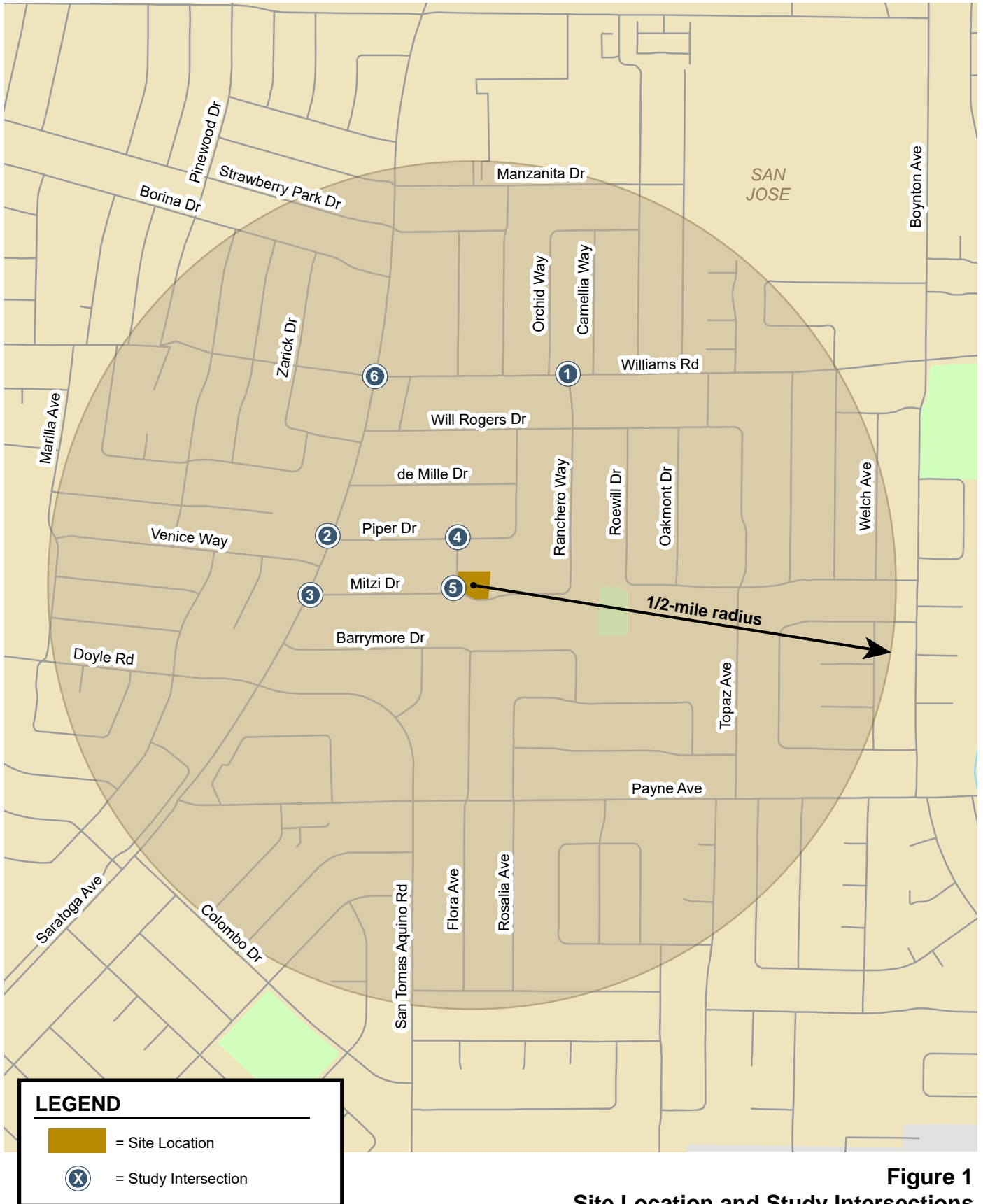
¹ Delays based on worst approach delay for unsignalized intersections and average delay for signalized intersections.
Bold indicates a substandard level of service.

1. Introduction

This report presents the results of the transportation analysis conducted for the proposed residential development located at 4146 Mitzi Road in San Jose, California (see Figure 1). The proposed project would consist of 46 apartments comprising a 40-unit building and conversion of the existing single-family residence on the site into a multi-family building with 6 units (see Figure 2). The project proposes a single driveway on Mitzi Lane leading to an underground parking garage serving all 46 units. This study was conducted for the purpose of satisfying the requirements of the California Environmental Quality Act (CEQA) and the City of San Jose. The potential impacts of the project were evaluated in accordance with the standards set forth by the City of San Jose.

Scope of Study

This study was conducted for the purpose of identifying the potential transportation impacts related to the proposed development. The potential impacts of the project were evaluated in accordance with the standards set forth by the City of San Jose and the Santa Clara Valley Transportation Authority CMP. A County Congestion Management Program (CMP) analysis was not required because the project would generate fewer than 100 peak hour trips.



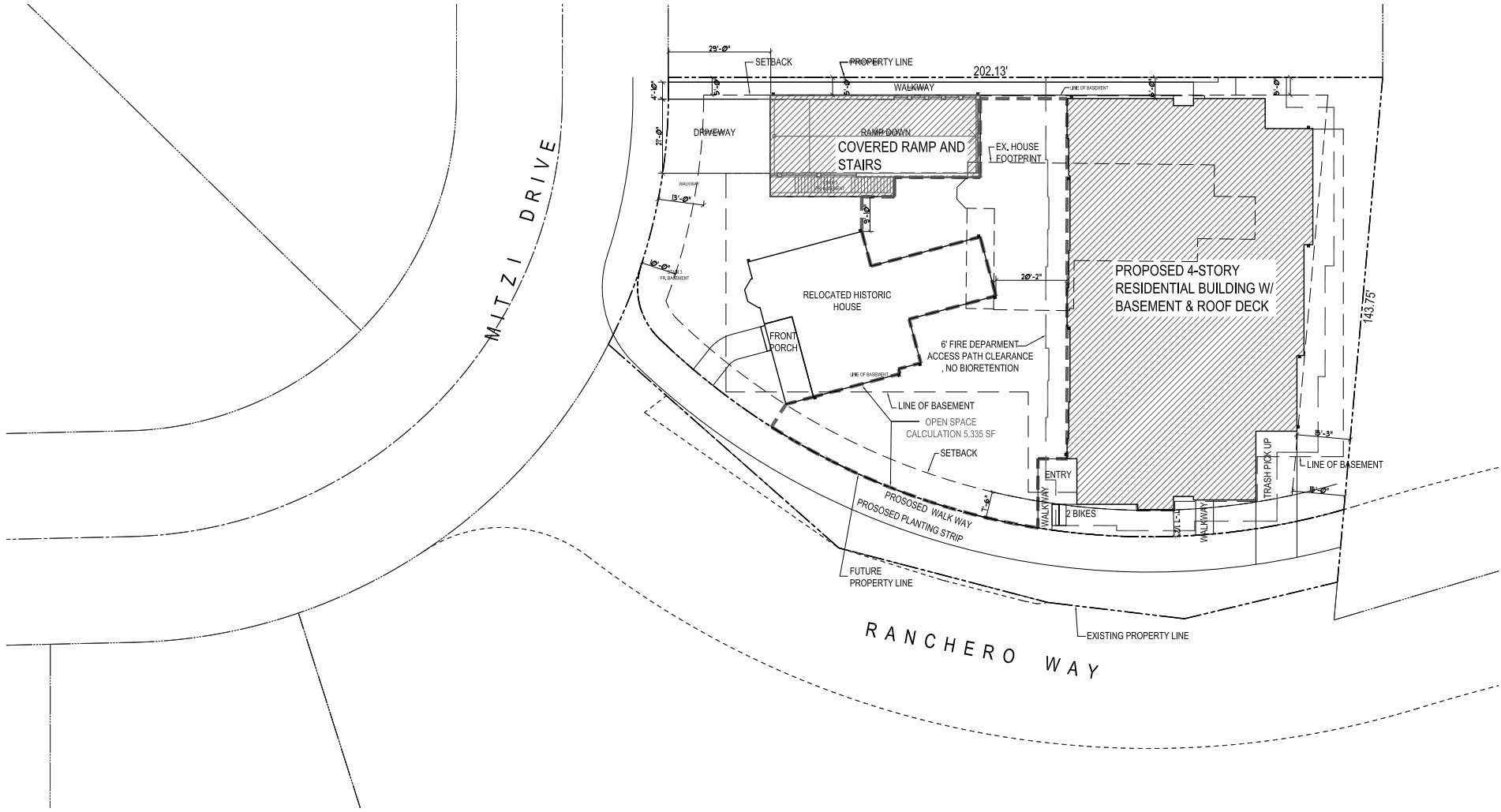


Figure 2
Site Plan

Transportation Policies

To align the City of San Jose's transportation analysis guidelines with State of California Senate Bill 743 (SB 743) and the City's goals as set forth in the Envision San Jose 2040 General Plan, the City of San Jose's adopted a new Transportation Analysis Policy (Council Policy 5-1) to replace the Transportation Level of Service Policy (Council Policy 5-3). The new policy establishes the thresholds for transportation impacts under California Environmental Quality Act (CEQA) based on vehicle miles traveled (VMT) instead of intersection level of service (LOS). The intent of this change is to shift the focus of transportation analysis under CEQA from vehicle delay and roadway auto capacity to a reduction in vehicle emissions, and the creation of robust multimodal networks that support integrated land uses. The new Transportation Analysis Policy took effect on March 29, 2018.

The new Transportation Analysis Policy aligns with the Envision San Jose 2040 General Plan which seeks to focus new development growth within Planned Growth Areas, bringing together office, residential, and service land uses to internalize trips and reduce VMT. VMT-based policies support dense, mixed-use, infill projects as established in the General Plan's Planned Growth Areas. The Envision San Jose 2040 General Plan contains the following policies to encourage the use of non-automobile transportation modes to minimize vehicle trip generation and reduce VMT:

- Consider impacts on overall mobility and all travel modes when evaluating transportation impacts of new developments or infrastructure projects (TR-1.2);
- Through the entitlement process for new development, projects shall be required to fund, or construct needed transportation improvements for all transportation modes, giving first consideration to improvement of biking, walking and transit facilities and services that encourage reduced vehicle travel demand (TR-1.4);
- Require new development where feasible to provide on-site facilities such as bicycle storage and showers, provide connections to existing and planned facilities, dedicate land to expand existing facilities or provide new facilities such as sidewalks and/or bicycle lanes/paths, or share in the cost of improvements (TR-2.8);
- As part of the development review process, require that new development along existing and planned transit facilities consist of land use and development types and intensities that contribute towards transit ridership. In addition, require that new development is designed to accommodate and to provide direct access to transit facilities (TR-3.3);
- Discourage, as part of the entitlement process, the provision of parking spaces significantly above the number of spaces required by code for a given use (TR-8.4);
- Allow reduced parking requirements for mixed-use developments and for developments providing shared parking or a comprehensive transportation demand management (TDM) program, or developments located near major transit hubs or within Villages and Corridors and other growth areas (TR-8.6);
- Encourage private property owners to share their underutilized parking supplies with the general public and/or other adjacent private developments (TR-8.7);
- Within new development, create and maintain a pedestrian-friendly environment by connecting the internal components with safe, convenient, accessible, and pleasant pedestrian facilities and

by requiring pedestrian connections between building entrances, other site features, and adjacent public streets (CD-3.3);

- Create a pedestrian-friendly environment by connecting new residential development with safe, convenient, accessible, and pleasant pedestrian facilities. Provide such connections between new development, its adjoining neighborhood, transit access points, schools, parks, and nearby commercial areas (LU-9.1);
- Encourage all developers to install and maintain trails when new development occurs adjacent to a designated trail location. Use the City's Parkland Dedication Ordinance and Park Impact Ordinance to have residential developers build trails when new residential development occurs adjacent to a designated trail location, consistent with other parkland priorities. Encourage developers or property owners to enter into formal agreements with the City to maintain trails adjacent to their properties (PR-8.5).

CEQA Transportation Analysis Scope

The CEQA Transportation Analysis includes an evaluation of VMT and potential impacts on transportation facilities in other jurisdictions including intersections in the City of San Jose.

VMT Analysis

The City of San Jose's Transportation Analysis Policy establishes procedures for determining project impacts on VMT based on project description, characteristics, and/or location. The City of San Jose defines VMT as the total miles of travel by personal motorized vehicles a project is expected to generate in a day. VMT is calculated for residential, office, and industrial projects using the Origin-Destination VMT method, which measures the full distance of personal motorized vehicle-trips with one end within the project.

A project's VMT is compared to the appropriate thresholds of significance based on the project location and type of development. When assessing a residential project, the project's VMT is divided by the number of residents expected to occupy the project to determine the VMT per capita. When assessing an office or industrial project, the project's VMT is divided by the number of employees.

The thresholds of significance for development projects, as established in the Transportation Analysis Policy, are based on the existing citywide average VMT level for residential uses and the existing regional average VMT level for employment uses. Figures 3 and 4 show the current VMT levels estimated by the City for residents and workers, respectively, based on the locations of residences and jobs. Developments in the green-colored areas are estimated to have VMT levels that are below the thresholds of significance, while the pink-colored areas are estimated to have VMT levels that are above the thresholds of significance.

The CEQA transportation analysis of the project includes a project-level VMT impact analysis using the City's sketch tool and a cumulative impact analysis that demonstrates the project's consistency with the Envision San Jose 2040 General Plan.

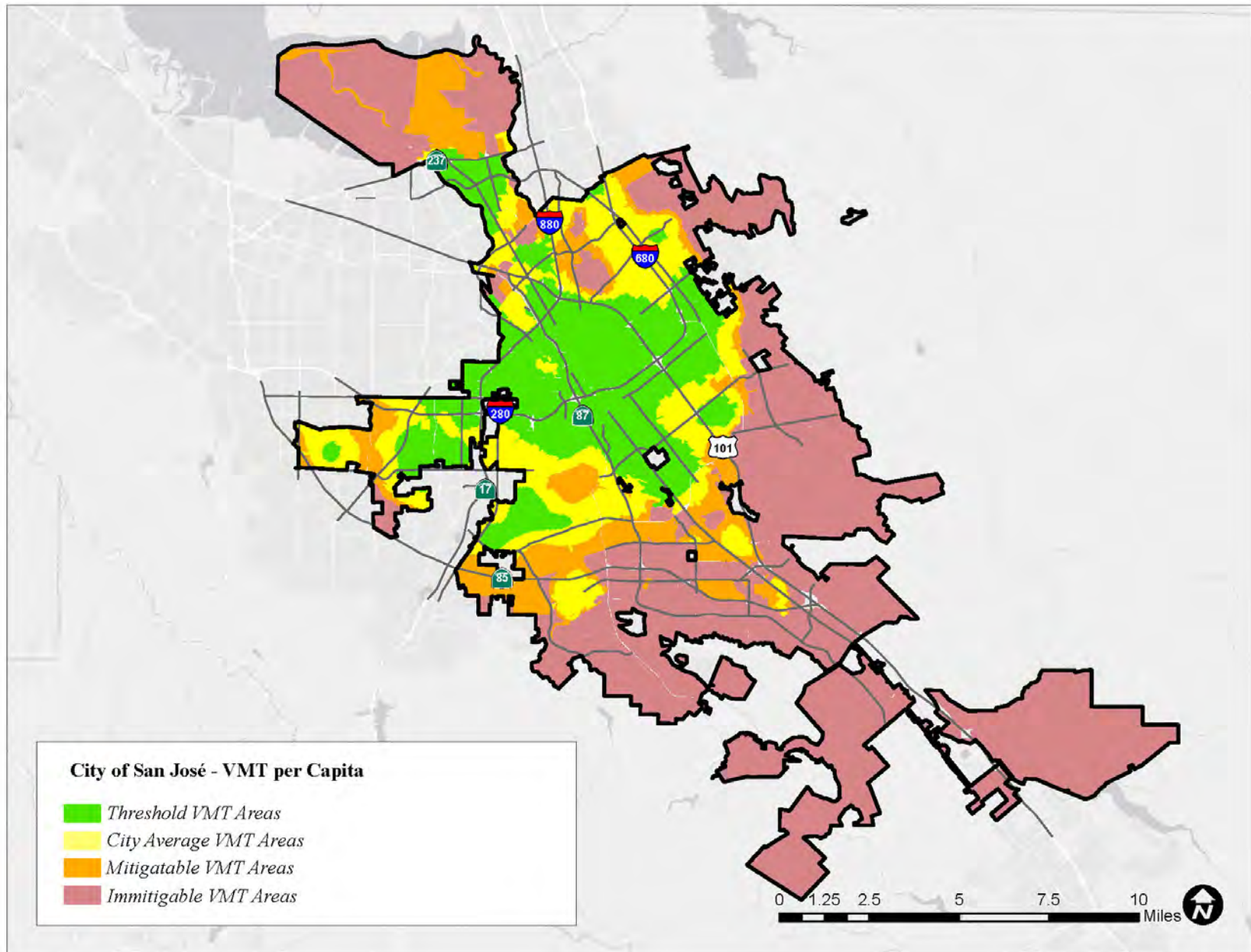
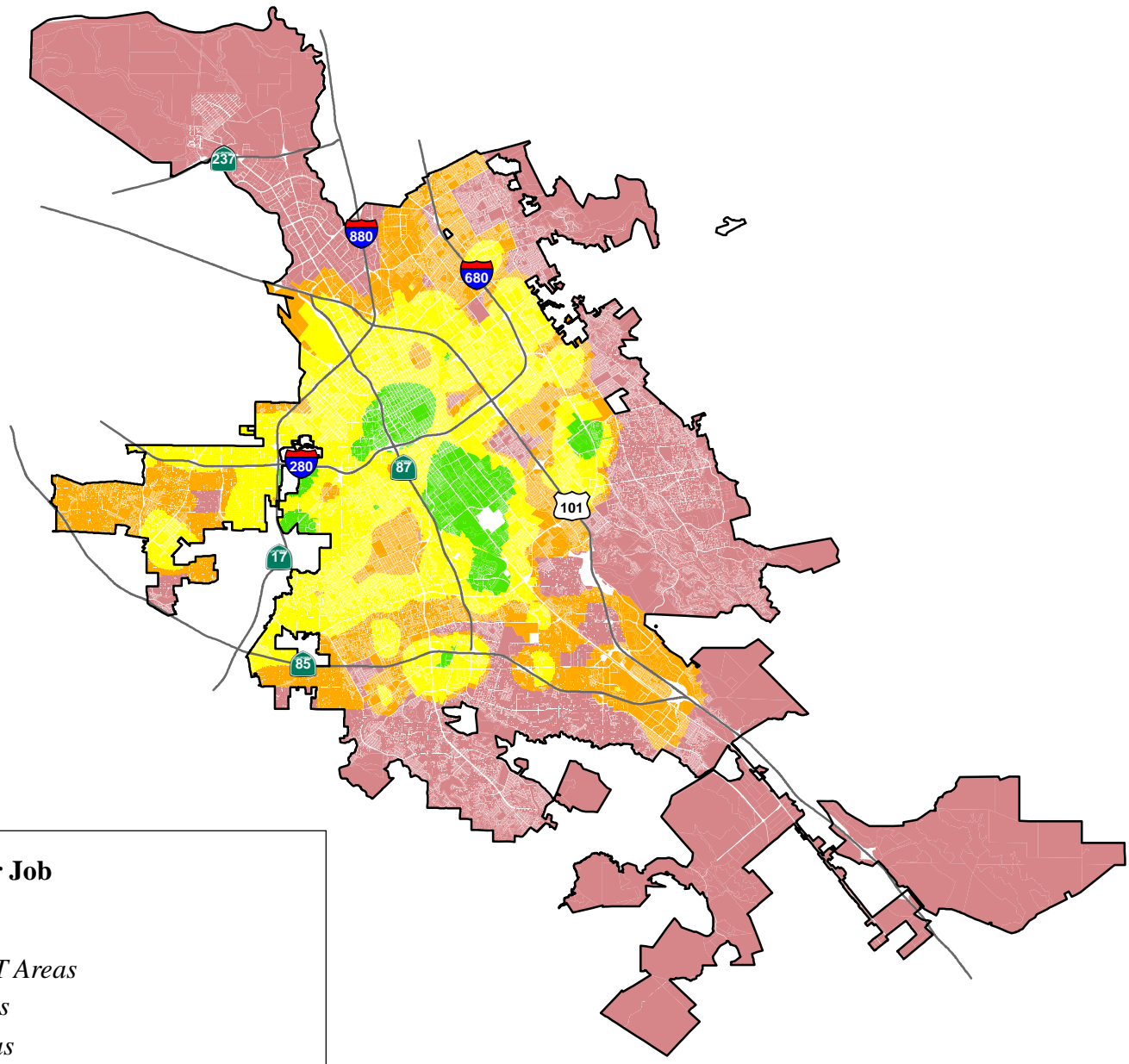


Figure 3
VMT Heat Map for Residents in San Jose



City of San José - VMT per Job

- Threshold VMT Areas*
- Regional Average VMT Areas*
- Mitigatable VMT Areas*
- Immitigable VMT Areas*

Figure 4
VMT Heat Map for Workers in San Jose

Screening for VMT Analysis

The *Transportation Analysis Handbook 2018* includes screening criteria for projects that are expected to result in less-than-significant VMT impacts. Projects that meet the screening criteria do not require a CEQA transportation analysis but may still be required to provide a Local Transportation Analysis (LTA).

The size of the proposed residential project does not qualify as a small infill project. Thus, the proposed project does not meet the screening criteria set forth in the *Transportation Analysis Handbook 2018*, and the project requires a detailed CEQA transportation analysis.

Local Transportation Analysis Scope

A local transportation analysis (LTA) identifies transportation operational issues that may arise due to a development project, evaluates the effects of the project on transportation, access, circulation, and related safety elements in the proximate area of the project, and supplements the VMT analysis.

As part of the LTA, a project is required to conduct an intersection operations analysis for any signalized intersections within a half mile of the project and at any signalized intersections currently operating at LOS D or worse within one mile of the project. Unsignalized intersections were also analyzed to determine whether a traffic signal or other operational improvements would be warranted based on the peak-hour volume signal warrant (Warrant #3) described in the *2014 California Manual on Uniform Traffic Control Devices* (CA MUTCD). Therefore, the LTA analysis evaluates AM and PM peak-hour traffic operations conditions for the following intersections.

- Saratoga Avenue and Williams Road
- Rancho Way and Williams Road (unsignalized)
- Saratoga Avenue and Piper Drive (unsignalized)
- Saratoga Avenue and Mitzi Drive (unsignalized)
- Piper Drive and Mitzi Drive (unsignalized)
- Rancho Way and Mitzi Drive (unsignalized)

Traffic conditions at the study intersections were analyzed for both the weekday AM and PM peak hours of adjacent street traffic. The AM peak hour is expected to occur between 7:00 AM and 9:00 AM and the PM peak hour is expected to occur between 4:00 PM and 6:00 PM on a regular weekday. These are the peak commute hours during which most traffic congestion occurs on the roadways.

Intersection operations conditions were evaluated for the following scenarios:

- **Existing Conditions.** Existing traffic volumes at the study intersections were obtained from new traffic counts conducted in November 2018.
- **Background Conditions.** Background traffic volumes reflect traffic added by nearby approved projects that are not yet completed or occupied. The added traffic from approved but not yet completed developments was provided by the City of San Jose.
- **Background Plus Project Conditions.** Background plus project conditions reflect projected traffic volumes on the planned roadway network with completion of the project and approved developments. Background plus project traffic volumes were estimated by adding to background traffic volumes the additional traffic generated by the project.

- **Near-term Cumulative Conditions.** Cumulative traffic volumes reflect projected traffic volumes projected to occur due to the approved development projects and other proposed but not yet approved (pending) development projects in the study area. Two proposed projects in the vicinity are as follows:
 - 700 Saratoga Avenue – A mixed-use residential development that includes 300 apartment units and 17,800 square feet (s.f.) of retail space.
 - 1777 Saratoga Avenue – A mixed-use project consisting of a 200-unit residential care facility for the elderly and a 35,000 square foot (s.f.) medical office building.

The LTA also includes a vehicle queuing analysis, an evaluation of potential project impacts on bicycle, pedestrian, and transit facilities, and a review of site access, on-site circulation, and parking demand.

VMT Analysis Methodology

Methodology

To determine whether a project would result in CEQA transportation impacts related to VMT, the City has developed the San Jose VMT Evaluation Tool (sketch tool) to streamline the analysis for residential, office, and industrial projects with local traffic. For larger projects with regional traffic, the City's Travel Demand Model can be used to determine project VMT. Because the proposed project is small and would generate local traffic, the sketch tool is used to estimate the project VMT and determine whether the project would result in a significant VMT impact.

Based on the assessor's parcel number (APN) of a project, the sketch tool identifies the existing average VMT per resident for the area. Based on the project location, type of development, project description, and proposed trip reduction measures, the sketch tool calculates the project VMT. Projects located in areas where the existing VMT is above the established threshold are referred to as being in "high-VMT areas". Projects in high-VMT areas are required to include a set of VMT reduction measures that would reduce the project VMT to the extent possible.

The sketch tool evaluates a list of selected VMT reduction measures that can be applied to a project to reduce the project VMT. There are four strategy tiers whose effects on VMT can be calculated with the sketch tool:

1. Project characteristics (e.g. density, diversity of uses, design, and affordability of housing) that encourage walking, biking and transit uses.
2. Multimodal network improvements that increase accessibility for transit users, bicyclists, and pedestrians,
3. Parking measures that discourage personal motorized vehicle-trips, and
4. Transportation demand management (TDM) measures that provide incentives and services to encourage alternatives to personal motorized vehicle-trips.

The first three strategies – land use characteristics, multimodal network improvements, and parking – are physical design strategies that can be incorporated into the project design. TDM includes programmatic measures that aim to reduce VMT by decreasing personal motorized vehicle mode share and by encouraging more walking, biking, and riding transit.

Thresholds of Significance

Table 1 shows the thresholds of significance for development projects, as established in the Transportation Analysis Policy. The threshold of significance for the proposed project is based on the existing regional average VMT per capita for residential uses.

Table 1
Thresholds of Significance for Development Projects (March 2018)

| Project Types | Significance Criteria | Current Level | Threshold |
|---|--|---|-----------------------------------|
| Residential Uses | Project VMT per capita exceeds existing citywide average VMT per capita minus 15 percent OR existing regional average VMT per capita minus 15 percent, whichever is lower. | 11.91 VMT per capita (Citywide Average) | 10.12 VMT per capita |
| General Employment Uses | Project VMT per employee exceeds existing regional average VMT per employee minus 15 percent. | 14.37 VMT per employee (Regional Average) | 12.21 VMT per employee |
| Industrial Employment Uses | Project VMT per employee exceeds existing regional average VMT per employee. | 14.37 VMT per employee (Regional Average) | 14.37 VMT per employee |
| Retail/ Hotel/ School Uses | Net increase in existing regional total VMT. | Regional Total VMT | Net Increase |
| Public/Quasi-Public Uses | In accordance with most appropriate type(s) as determined by Public Works Director. | Aproprate levels listed above | Aproprate thresholds listed above |
| Mixed-Uses | Evaluate each land use component of a mixed-use project independently, and apply the threshold of significance for each land use type included. | Aproprate levels listed above | Aproprate thresholds listed above |
| Change of Use/ Additions to Existing Development | Evaluate the full site with the change of use or additions to existing development, and apply the threshold of significance for each project type included. | Aproprate levels listed above | Aproprate thresholds listed above |
| Area Plans | Evaluate each land use component of the area plan independently, and apply the threshold of significance for each land use type included. | Aproprate levels listed above | Aproprate thresholds listed above |

Source: City of San Jose, 2018 *Transportation Analysis Handbook*, Table 2.

Intersection Operations Analysis Methodology

This section presents the methods used to determine the traffic operations conditions at the study intersections and the impacts of the project on intersection operations. It includes descriptions of the data requirements, the analysis methodologies, and the applicable level of service standards for identifying deficiencies.

Data Requirements

The data required for the analysis were obtained from new traffic counts, the City of San Jose, and field observations. The following data were collected from these sources:

- existing traffic volumes
- approved project trips
- existing lane configurations
- signal timing and phasing

Level of Service Standards and Analysis Methodologies

Traffic conditions at the study intersections were evaluated using level of service (LOS). *Level of Service* is a qualitative description of operating conditions ranging from LOS A, or free-flow conditions with little or no delay, to LOS F, or jammed conditions with excessive delays. The various analysis methods are described below.

City of San Jose Signalized Intersections

The City of San Jose level of service methodology for signalized intersections is the 2000 *Highway Capacity Manual* (HCM) method. This method is applied using the TRAFFIX software. The 2000 HCM operations method evaluates signalized intersection operations on the basis of average control delay time for all vehicles at the intersection. Since TRAFFIX is also the CMP-designated intersection level of service methodology, the City of San Jose methodology employs the CMP default values for the analysis parameters. The City of San Jose level of service standard for signalized intersections is LOS D or better, whether or not the intersection is a CMP intersection. The correlation between average control delay and level of service is shown in Table 2.

City of San Jose Unsignalized Intersections

San Jose does not have a level of service standard for unsignalized intersections. The unsignalized study intersections were evaluated for operational issues. The unsignalized study intersection also were assessed to determine whether a traffic signal would be warranted based on the peak-hour volume signal warrant (Warrant #3) described in the *2014 California Manual on Uniform Traffic Control Devices* (CA MUTCD). This method provides an indication of whether traffic conditions and peak-hour traffic levels are, or would be, sufficient to justify installation of a traffic signal. Note that this is just one tool used to evaluate whether installation of a traffic signal would be justified. Intersections that meet the peak-hour warrant are subject to further analysis before determining that a traffic signal is necessary. Other types of traffic control devices, signage, or geometric changes may be preferable at unsignalized locations based on existing field conditions. The following unsignalized intersections were analyzed:

1. Rancho Way and Williams Road
2. Saratoga Avenue and Piper Drive
3. Saratoga Avenue and Mitzi Drive
4. Piper Drive and Mitzi Drive
5. Rancho Way and Mitzi Drive

Table 2
Signalized Intersection of Level of Service Definitions Based on Control Delay

| Level of Service | Description | Average Control Delay Per Vehicle (Sec.) |
|------------------|---|--|
| A | Operations with very low delay occurring with favorable progression and/or short cycle lengths. | Up to 10.0 |
| B | Operations with low delay occurring with good progression and/or short cycle lengths. | 10.1 to 20.0 |
| C | Operation with average delays resulting from fair progression and/or longer cycle lengths. Individual cycle failures begin to appear. | 20.1 to 35.0 |
| D | Operations with longer delays due to a combination of unfavorable progression, long cycle lengths or high V/C ratios. Many vehicles stop and individual cycle failures are noticeable. | 35.1 to 55.0 |
| E | Operations with high delay values indicating poor progression, long cycle lengths, and high V/C ratios. Individual cycle failures are frequent occurrences. This is considered to be the limit of acceptable delay. | 55.1 to 80.0 |
| F | Operations with delays unacceptable to most drivers occurring due to over saturation, poor progression, or very long cycle lengths. | Greater than 80.0 |

Source: Transportation Research Board, *2000 Highway Capacity Manual*, (Washington, D.C., 2000)

Intersection Vehicle Queuing Analysis

For selected high-demand movements at the study intersections, the estimated maximum vehicle queues were compared to the existing or planned storage capacity. The queuing analysis is presented for informational purposes only, since the City of San Jose has not defined a policy related to queuing. Vehicle queues were calculated using a Poisson probability distribution, which estimates the probability of “n” vehicles for a vehicle movement using the following formula:

$$P(x = n) = \frac{\lambda^n e^{-\lambda}}{n!}$$

Where:

P (x = n) = probability of “n” vehicles in queue per lane

n = number of vehicles in the queue per lane

λ = average # of vehicles in the queue per lane (vehicles per hr. per lane/signal cycles per hr.)

The basis of the analysis is as follows: (1) the Poisson probability distribution is used to estimate the 95th percentile maximum number of queued vehicles per signal cycle for a particular movement; (2) the estimated maximum number of vehicles in the queue is translated into a queue length, assuming 25 feet per vehicle; and (3) the estimated maximum queue length is compared to the existing or planned available storage capacity for the movement.

For signalized intersections, the 95th percentile queue length value indicates that during the peak hour, a queue of this length or less would occur on 95 percent of the signal cycles. Or, a queue length larger than the 95th percentile queue would only occur on 5 percent of the signal cycles (about 3 cycles during

the peak hour for a signal with a 60-second cycle length). Therefore, left-turn pocket storage designs based on the 95th percentile queue length would ensure that storage space would be exceeded only 5 percent of the time for a signalized movement. Vehicle queuing at unsignalized intersections is evaluated based on the delay experienced at the specific study turn movement.

Adverse Intersection Operations Effects

The criteria used to determine intersection operations effects on signalized intersections are based on City of San Jose Level of Service standards.

The project is said to create a deficiency at a signalized intersection in the City of San Jose if for either peak hour:

1. The level of service at the intersection degrades from an acceptable LOS D or better under background conditions to an unacceptable LOS E or F under background plus project conditions, or
2. The level of service at the intersection is an unacceptable LOS E or F under background conditions and the addition of project trips cause both the critical-movement delay at the intersection to increase by four (4) or more seconds and the volume-to-capacity ratio (V/C) to increase by one percent (.01) or more.

An exception to rule #2 above applies when the addition of project trips reduces the amount of average delay for critical movements (i.e., the change in average delay for critical movements is negative). In this case, a deficiency is identified if there is an increase in the critical V/C value by .01 or more.

Report Organization

This report has a total of five chapters. Chapter 2 describes existing transportation conditions including VMT of the existing land uses in the proximity of the project, the existing roadway network, transit service, bicycle and pedestrian facilities. Chapter 3 describes the CEQA transportation analysis, including the project VMT and cumulative transportation analysis. Chapter 4 describes the local transportation analysis including the method by which project traffic is estimated, intersection operations analysis for background plus project conditions, any adverse intersection traffic effects caused by the project, intersection vehicle queuing analysis, site access and on-site circulation review, effects on bicycle, pedestrian, and transit facilities, and parking. Chapter 5 presents the conclusions of the transportation analysis.

2. Existing Conditions

This chapter describes the existing conditions for transportation facilities in the vicinity of the site, including the roadway network, transit service, pedestrian and bicycle facilities, and the existing levels of service of the key intersections in the study area.

VMT of Existing Land Uses

To determine whether a project would result in CEQA transportation impacts related to VMT, the City has developed the San Jose VMT Evaluation Tool (sketch tool) to streamline the analysis for residential, office, and industrial projects.

Based on the sketch tool and the project's APN, the existing VMT for residential uses in the project vicinity is 8.52 per capita. As shown in Table 1, the current citywide average VMT for residential use is 11.91 per capita. Therefore, the VMT levels of existing uses in the project vicinity are less than the average VMT levels. Figure 5 shows the heat map of the project site. Appendix A presents the sketch tool summary report for the project.

Existing Roadway Network

Local roadways in the vicinity of the site include Saratoga Avenue, Mitzi Drive, Rancho Way, Williams Road, and Piper Drive. These roadways are described below.

Saratoga Avenue is a north-south arterial roadway that extends between Saratoga-Sunnyvale Road/Los Gatos-Saratoga Road to the south and Fallon Avenue to the north. Saratoga Avenue has a posted speed limit of 40 mph and is six lanes wide in the project vicinity.

Mitzi Drive is a local roadway that extends east-west between Saratoga Drive and Rancho Way and bends north-south between Rancho Way and Piper Drive. Mitzi Drive has a posted speed limit of 25 mph and is two lanes wide. Mitzi Drive generally has sidewalks on both sides of the street, but discontinuous sidewalks exist on one side of Mitzi Drive near the project site.

Rancho Way is a local roadway that extends between Mitzi Drive and Williams Road. Rancho Way has a posted speed limit of 25 mph and is two lanes wide. Rancho Way runs east-west and then runs north-south at a 90-degree bend approximately 700 feet east of Mitzi Drive. Rancho Way has a constrained width along the project frontage, and parking is not allowed. Also, sidewalks do not exist along the project frontage.

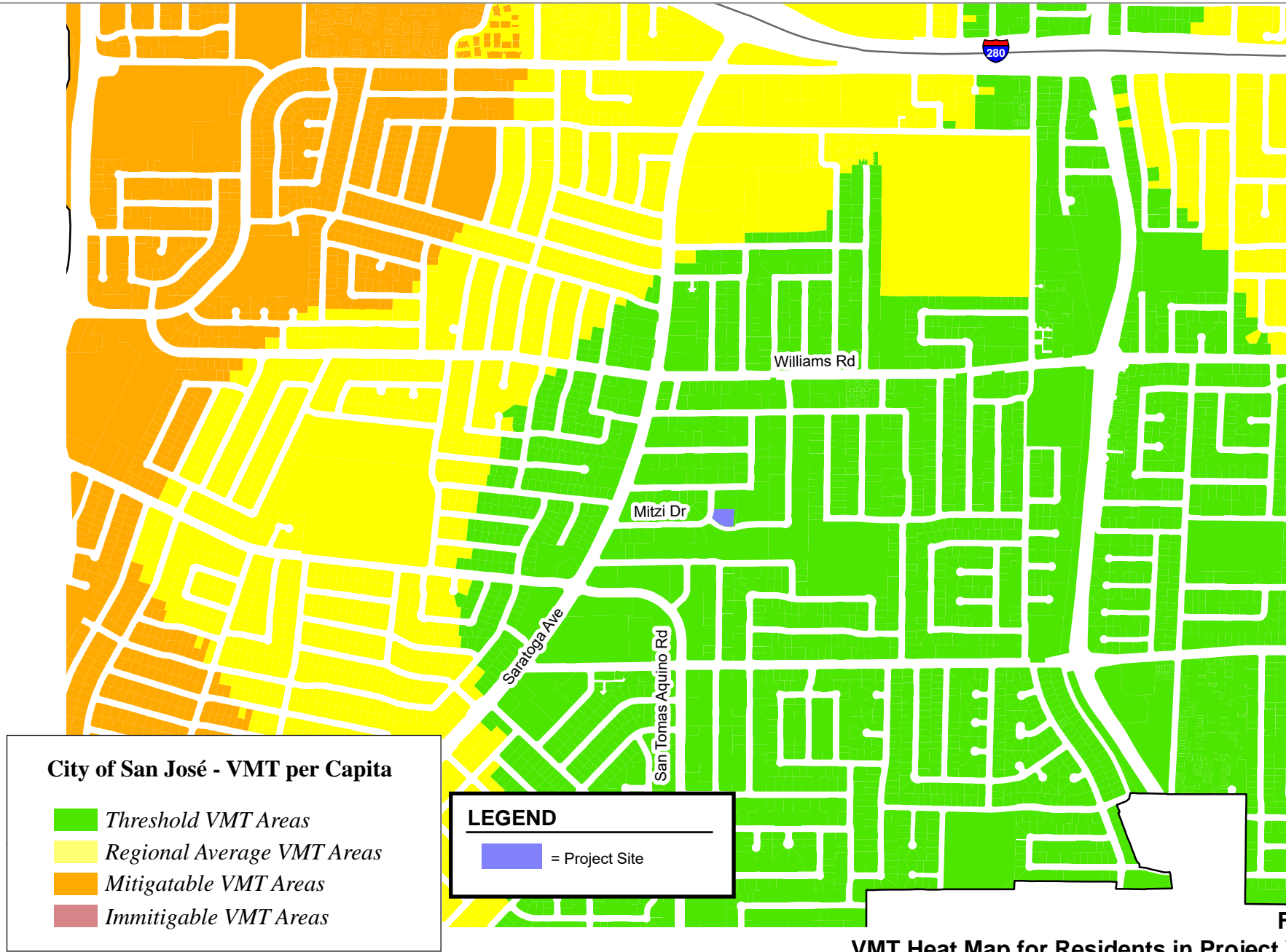


Figure 5
VMT Heat Map for Residents in Project Vicinity

Williams Road is an arterial roadway that extends between Moorpark Avenue to the west and Daniel Way to the east. Williams Road has a posted speed limit of 35 mph and is two lanes wide with a two-way left-turn lane in the project vicinity. In the vicinity of the project site, Williams Road provides bike lanes in both directions and has sidewalks and on-street parking allowed on both sides of the street.

Piper Drive is a local roadway that extends between Saratoga Avenue to the west and Leslie Drive to the east. Piper Drive has a posted speed limit of 25 mph and is two lanes wide. There are sidewalks and on-street parking on both sides of the street.

Existing Pedestrian and Bicycle Facilities

Pedestrian facilities consist of sidewalks, crosswalks, and pedestrian signals at signalized intersections. In the project vicinity, sidewalks exist along all nearby streets except along the project frontage. Marked crosswalks with pedestrian signal heads and push buttons are provided at the signalized intersection of Saratoga Avenue and Williams Road. Overall, the existing network of sidewalks and crosswalks in the immediate vicinity of the project site has good connectivity and provides pedestrians with safe routes to other points of interest in the study area, including nearby bus stops on Saratoga Avenue and Williams Road.

In the vicinity of the project, bike lanes (Class II Bikeway) exist on Williams Road from Moorpark Avenue to Daniel Way.

Existing Transit Service

Transit service to the study area is provided by the Santa Clara Valley Transportation Authority (VTA). This is described below.

Local Route 25 provides service between De Anza College and the Alum Rock Transit Center via Valley Medical Center. Route 25 operates along Williams Road in the project study area, with 20-minute headways during the weekday peak commute hours and 30-minute headways during most of the days on weekends. The closest bus stop is located on Williams Road near Saratoga Avenue.

Local Route 57 provides service between West Valley College and Great America. Route 57 operates along Saratoga Avenue in the project study area, with 30-minute headways during the weekday peak commute hours and weekends. The closest bus stop is located on Saratoga Avenue near Mitzi Drive.

Local Route 58 provides service between West Valley College and Alviso. Route 58 operates along Saratoga Avenue in the project study area, with 30-minute headways during the weekday peak commute hours. The closest bus stop is located on Saratoga Avenue near Mitzi Drive.

Existing Intersection Lane Configurations

The existing lane configurations at the study intersections were determined by observations in the field and are shown on Figure 6
Existing Lane Configurations .

Existing Traffic Volumes

Existing AM and PM peak hour traffic volumes for the six study intersections were obtained from new manual turning-movement counts conducted in November 2018. The existing peak-hour intersection

volumes are shown in Figure 7. Intersection turning-movement counts conducted for this analysis are presented in Appendix B.

Existing Intersection Traffic Operations

Intersection traffic operations were evaluated against City of San Jose standards. The results of the intersection level of service analysis under existing conditions are summarized in Table 3.

The results of the analysis show that the Saratoga Avenue and Williams Road intersection operates at an acceptable level of service (LOS D or better) during the AM and PM peak hour.

The results of the analysis show that the left turn from Piper Drive to Saratoga Avenue operates at LOS F during the AM peak hour. However, the City of San Jose does not have a level of service standard for unsignalized intersections. The traffic counts showed that there were only 17 vehicles making the left-turn movement during the AM peak hour. Field observations showed a maximum queue of 2 vehicles making the left turn from Piper Drive onto Saratoga Avenue during the AM peak hour.

The intersection levels of service calculation sheets are included in Appendix C.

Mitzi Drive Residential

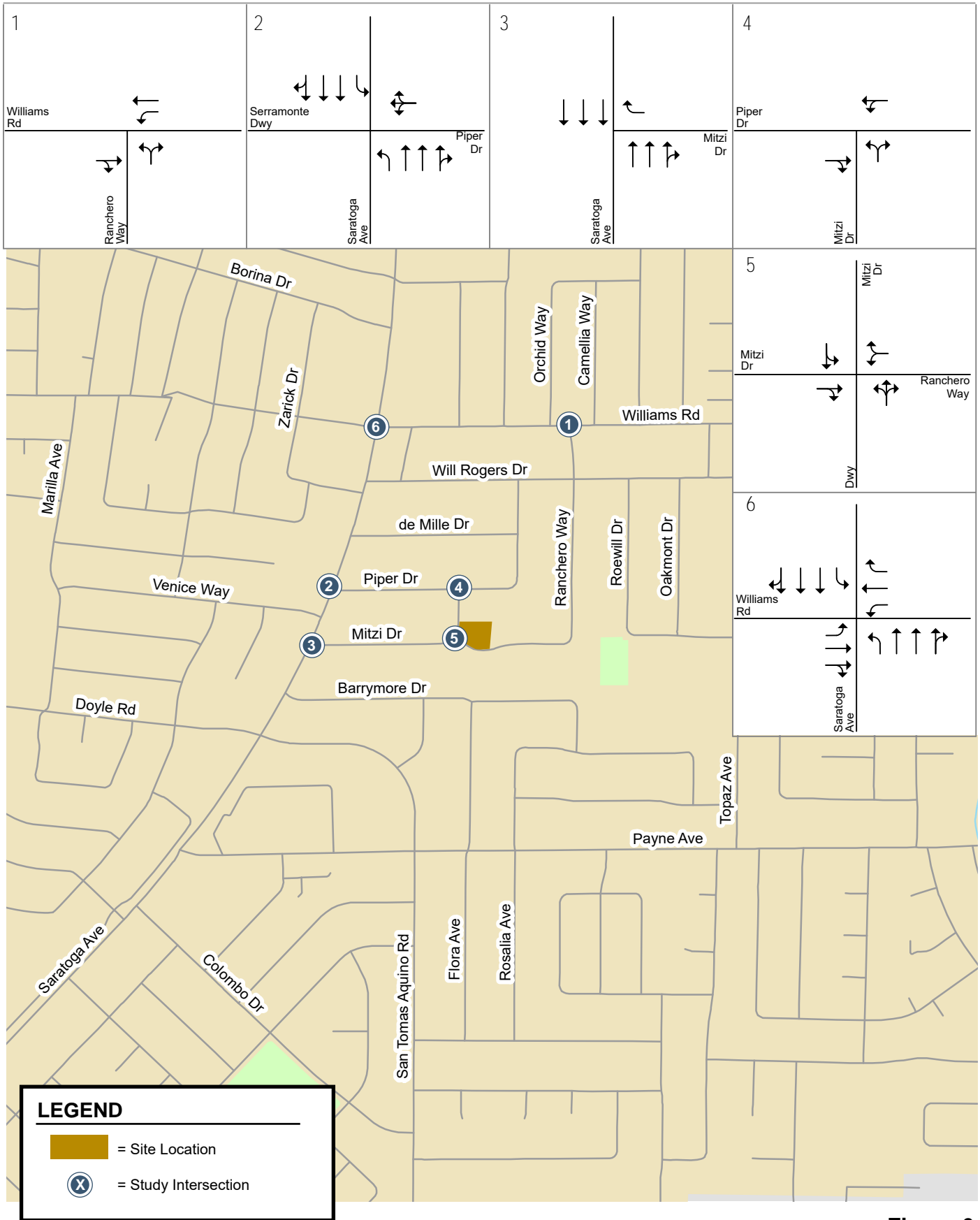


Figure 6
Existing Lane Configurations

Mitzi Drive Residential

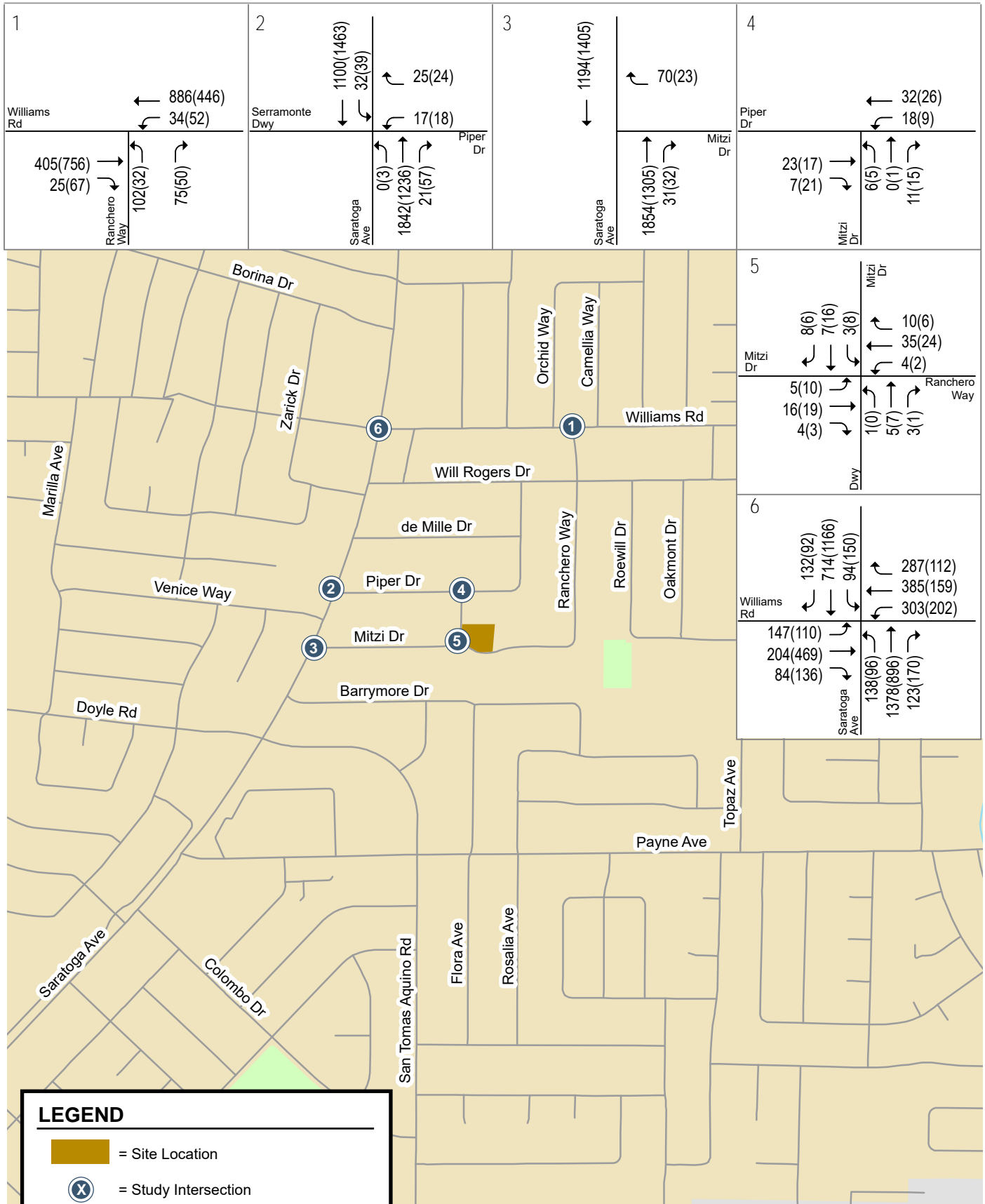


Figure 7
Existing Traffic Volumes

Table 3
Existing Intersection Levels of Service

| Study Number | Intersection | Peak Hour | Count Date | Existing Conditions | |
|--------------|--|-----------|------------|--------------------------|----------|
| | | | | Delay (sec) ¹ | LOS |
| 1 | Ranchero Way and Williams Road (unsignalized) | AM | 11/06/18 | 23.6 | C |
| | | PM | 11/06/18 | 21.6 | C |
| 2 | Saratoga Avenue and Piper Drive (unsignalized) | AM | 11/06/18 | >90 | F |
| | | PM | 11/06/18 | 41.3 | E |
| 3 | Saratoga Avenue and Mitzi Drive (unsignalized) | AM | 11/06/18 | 15.1 | B |
| | | PM | 11/06/18 | 11.7 | B |
| 4 | Piper Drive and Mitzi Drive (unsignalized) | AM | 11/06/18 | 8.7 | A |
| | | PM | 11/06/18 | 8.6 | A |
| 5 | Ranchero Way and Mitzi Drive (unsignalized) | AM | 11/06/18 | 9.2 | A |
| | | PM | 11/06/18 | 9.2 | A |
| 6 | Saratoga Avenue and Williams Road | AM | 11/06/18 | 36.9 | D |
| | | PM | 11/06/18 | 41.1 | D |

Note:
¹ Delays based on worst approach delay for unsignalized intersections and average delay for signalized intersections.
Bold indicates a substandard level of service.

Observed Existing Traffic Conditions

Traffic conditions were observed in the field to identify existing operational deficiencies and to confirm the accuracy of calculated levels of service. The purpose of this effort was (1) to identify any existing traffic problems that may not be directly related to level of service, and (2) to identify any locations where the level of service analysis does not accurately reflect actual existing traffic conditions. AM and PM field observations conducted in October 2018 revealed that overall the study intersections operate well, and the level of service calculations accurately reflect existing conditions. However, operational issues were observed during the commute peak hours as described below.

Saratoga Avenue & Williams Road

During the AM peak hour, traffic in the westbound through direction spills back to Desert Isle Drive, approximately 730 feet east of Saratoga Avenue. Westbound through traffic requires multiple cycles to cross the intersection. The westbound left turn traffic also spills back to the next intersection, Larson Way, which is approximately 150 feet east of Saratoga Avenue. However, the westbound left turn queue does clear with each signal cycle.

During the PM peak hour, traffic in the eastbound through direction occasionally requires multiple cycles to cross the intersection. These conditions are typical of LOS D operations.

3.

CEQA Transportation Analysis

This chapter describes the CEQA transportation analysis, including the project VMT analysis and cumulative transportation analysis.

Project-Level VMT Analysis

The project-level impact analysis under CEQA uses the VMT metric to evaluate a project's transportation impacts by comparing against the VMT thresholds of significance as established in the Transportation Analysis Policy. The San Jose VMT Evaluation Tool (sketch tool) is used to estimate the project VMT, based on the project location (APN), type of development, project description, and proposed trip reduction measures, if any. The threshold of significance for residential uses (see Table 1) is used for the VMT analysis. The VMT threshold is 10.12 VMT per resident, which is a 15% reduction from the existing citywide average VMT level.

The project VMT estimated by the sketch tool is 8.52 per capita. The project VMT would not exceed the threshold of 10.12 VMT per capita. Therefore, the project's VMT impact is considered less than significant. Appendix A presents the sketch tool summary report for the project.

Cumulative Impact Analysis

Projects must demonstrate consistency with the Envision San Jose 2040 General Plan to address cumulative impacts. Consistency with the City's General Plan is based on the project's density, design, and conformance to the General Plan goals and policies. The project is consistent with the General Plan goals and policies for the following reasons:

- The project site is near bus stops on Saratoga Avenue and Williams Road and is near bicycle lanes on Williams Road.
- The project would increase the residential density in the project area.
- The project would provide bicycle parking.

Therefore, the project would be considered as part of the cumulative solution to meet the General Plan's long-range transportation goals and would result in a less-than-significant cumulative impact.

4. Local Transportation Analysis

This chapter describes the local transportation analysis including the method by which project traffic is estimated, intersection operations analysis for background plus project conditions, any adverse intersection traffic effects caused by the project, intersection vehicle queuing analysis, site access and on-site circulation review, effects on bicycle, pedestrian, and transit facilities, and parking.

Intersection Operations Analysis

The San Jose Intersection analysis methodology and standards are described in Chapter 1.

Project Trip Estimates

Trip Generation

Through empirical research, data have been collected that quantify the amount of traffic produced by many different types of land use. There are standard trip generation rates that can be applied to help predict the future traffic increases that would result from a new development. The magnitude of traffic added to the roadway system by a particular development is estimated by multiplying the applicable trip generation rates by the size of the development. Trip generation rates for new development proposed within the City of San Jose typically are estimated using the trip rates published in the Institute of Transportation Engineers' (ITE) *Trip Generation Manual*, 10th Edition. The ITE trip generation rates for Single-Family Detached Housing (Land Use 210) and Mid-Rise Multi-Family Housing (Land Use 221) were utilized for the proposed project.

Table 4 shows that, before reductions, the project would generate 249 gross daily vehicle trips, with 16 gross trips occurring during the AM peak hour and 21 gross trips occurring during the PM peak hour.

Trip Adjustments and Reductions

In accordance with San Jose's *Transportation Analysis Handbook* (April 2018, Section 4.8, "Intersection Operations Analysis"), the project is eligible for adjustments and reductions from the baseline trip generation described above. Based on the 2018 San Jose guidelines, the project qualifies for a location-based adjustment. The location-based adjustment reflects the project's vehicle mode share based on the place type in which the project is located per the San Jose Travel Demand Model. The project's place type was obtained from the *San Jose VMT Evaluation Tool*. Based on the Tool, the project site is located within a designated suburban with multifamily homes area. Therefore, the baseline project trips were adjusted to reflect a suburban multifamily homes mode share. According to the handbook Table 6, residential developments within suburban with multifamily homes areas have a

vehicle mode share of 88 percent. Thus, a 12 percent reduction was applied to the multifamily housing trips generated by the proposed project.

According to the *Transportation Analysis Handbook*, the VMT reduction resulting from implementing the VMT reduction strategies in the sketch tool should be included as part of the trip generation estimates. A 0.5% reduction was applied based on the project-specific VMT reductions obtained from the City's VMT sketch tool.

The existing single-family home can be credited against the proposed multifamily housing development. The existing home's trip generation was based on the average rates published by ITE. Based on ITE rates, it is estimated that the existing home is generating 9 daily trips with 1 trip occurring in the AM peak hour and 1 trip occurring in the PM peak hour.

Net Project Trips

After applying the ITE trip rates, appropriate trip reductions, and existing site trip credits, the project would generate 209 new daily vehicle trips, with 14 new trips occurring during the AM peak hour and 17 new trips occurring during the PM peak hour. Using the inbound/outbound splits contained in ITE's *Trip Generation Manual*, the project would produce 4 inbound and 10 outbound trips during the AM peak hour, and 10 inbound and 7 outbound trips during the PM peak hour (See Table 4).

**Table 4
Project Trip Generation Estimates**

| Land Use | Size | Unit | Daily | | | AM Peak Hour | | | PM Peak Hour | | | |
|---|------|------|-------|------------|------|--------------|-----------|-----------|--------------|-----------|----------|-----------|
| | | | Rate | Trips | Rate | In | Out | Total | Rate | In | Out | Total |
| Proposed Uses | | | | | | | | | | | | |
| Multifamily Housing ¹ | 46 | DU | 5.41 | 249 | 0.35 | 4 | 12 | 16 | 0.46 | 13 | 8 | 21 |
| <i>Location-based Adjustments (Suburban with Multifamily Homes - 12%)²</i> | | | | (30) | | 0 | (1) | (1) | | (2) | (1) | (3) |
| <i>Project-Specific Adjustments (0.5%)³</i> | | | | (1) | | 0 | 0 | 0 | | 0 | 0 | 0 |
| Total | | | | 218 | | 4 | 11 | 15 | | 11 | 7 | 18 |
| Existing Use | | | | | | | | | | | | |
| Single Family Home ⁴ | 1.0 | DU | 9.44 | (9) | 0.74 | 0 | (1) | (1) | 0.99 | (1) | 0 | (1) |
| Net Project Trips | | | | 209 | | 4 | 10 | 14 | | 10 | 7 | 17 |

Notes:

Trip rates for multifamily uses are from the ITE Trip Generation Manual, 10th Edition, 2017.

1. Mid-Rise Multifamily Housing (Land Use 221), average rates expressed in trips per dwelling unit (DU) are used.

2. A 12% reduction was applied based on the location-based vehicle mode share percentage outputs (Table 6 of the City's Transportation Analysis Handbook, 2018) produced from the San Jose Travel Demand Model for the place type Suburban with Multifamily Homes.

3. A 0.5% reduction was applied based on the VMT reductions obtained from the City's VMT sketch tool.

4. Single-Family Detached Housing (Land Use 210), average rates expressed in trips per dwelling unit (DU) are used.

Trip Distribution and Assignment

The trip distribution patterns for the project were estimated based on existing travel patterns on the surrounding roadway system and the locations of complementary land uses (see Figure 8). The peak-hour vehicle trips associated with the project were added to the roadway network in accordance with the trip distribution pattern, the roadway network connections, and the locations of project driveways (see Figure 9). All project trips would enter and exit the project site via a driveway on Mitzi Drive.

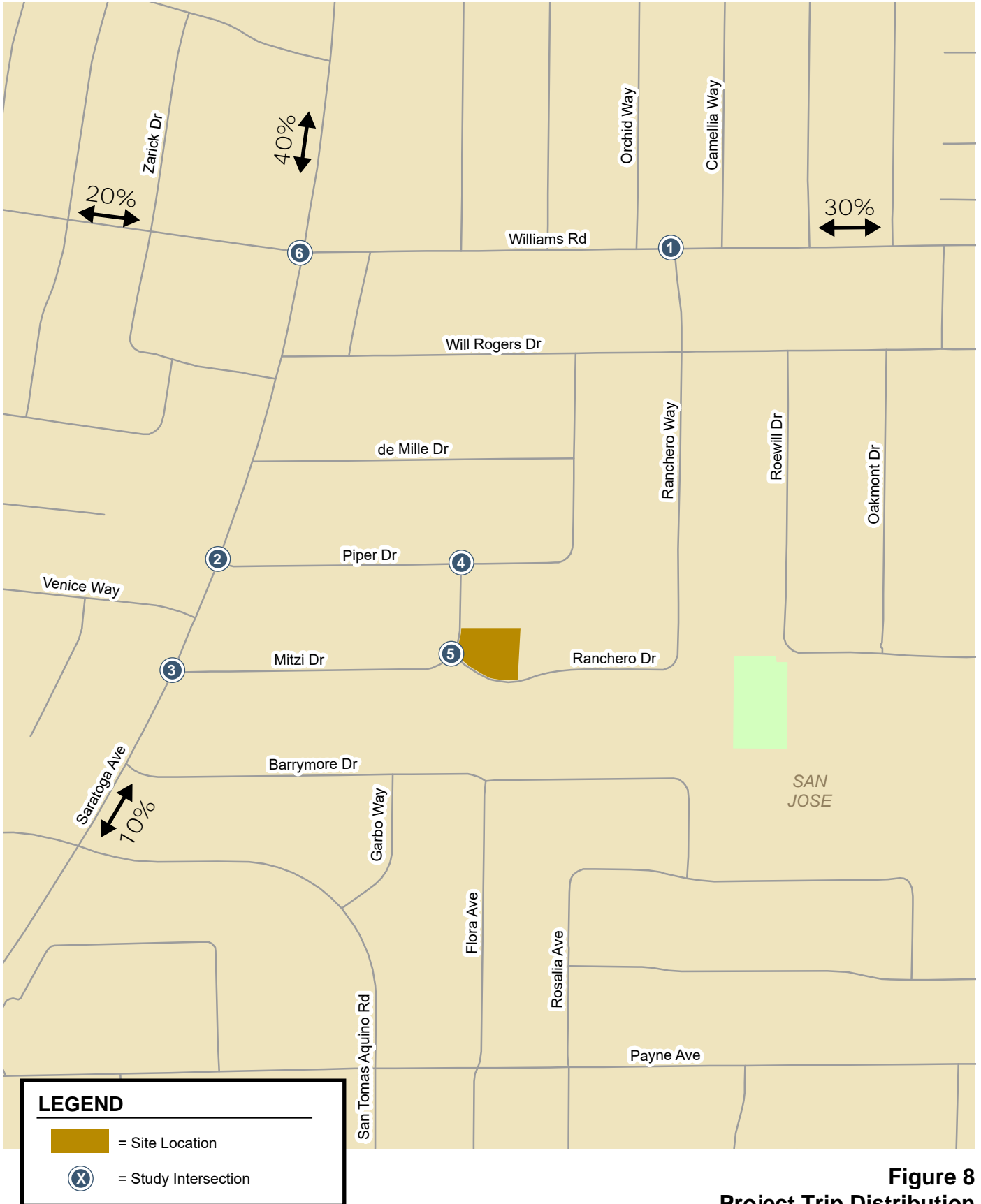
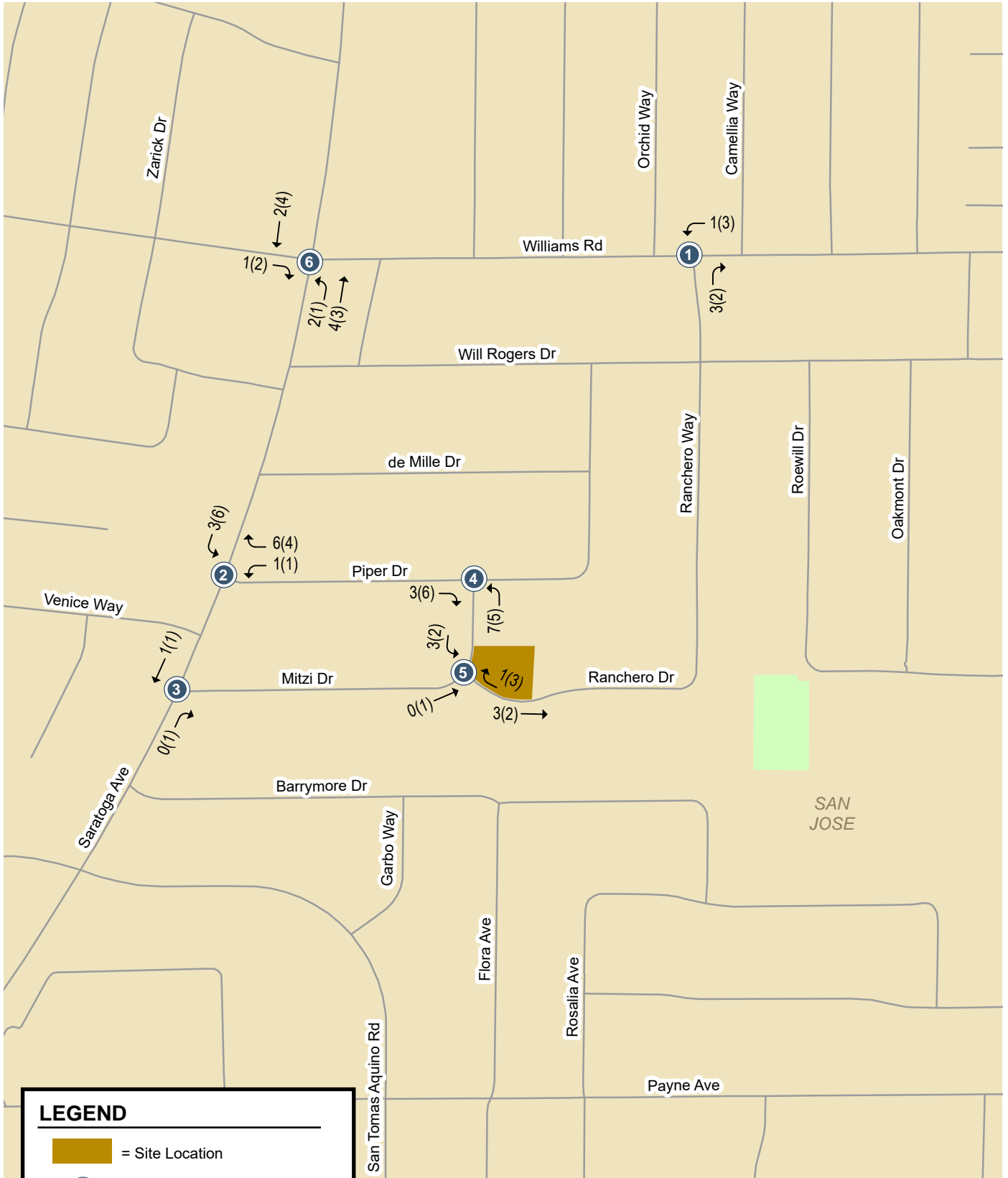


Figure 8
Project Trip Distribution



LEGEND

- = Site Location
- X = Study Intersection
- XX(XX) = AM(PM) Peak-Hour Trips

Figure 9
Project Trip Assignment

Roadway Network under Background and Project

The roadway network under background conditions and background plus project conditions would be the same as the existing roadway network because: 1) there are no approved projects in the area that would alter the existing roadway network, and 2) the project would not alter the existing roadway network.

Traffic Volumes under Background and Project Conditions

Background peak hour traffic volumes were estimated by adding to existing volumes the estimated traffic from approved but not yet constructed developments. The approved but not yet constructed trips are included in Appendix E. The added traffic from approved but not yet constructed developments was obtained from the City of San Jose. Background traffic volumes are shown on Figure 10.

Project trips were added to background traffic volumes to obtain background plus project traffic volumes (see Figure 11).

Traffic Volumes under Near-Term Cumulative Conditions

There are two proposed developments in the project vicinity that, if approved and built, would add traffic to some of the study intersections. These proposed developments are as follows:

- 700 Saratoga Avenue – A mixed-use residential development that includes 300 apartment units and 17,800 square feet (s.f.) of retail space.
- 1777 Saratoga Avenue – A mixed-use project consisting of a 200-unit residential care facility for the elderly and a 35,000 square foot (s.f.) medical office building.

The trips generated by these projects were estimated using the trip rates published in the Institute of Transportation Engineers' (ITE) *Trip Generation Manual*, 10th Edition, and added to the Background volumes. Near-term cumulative traffic volumes are shown on Figure 12.

Mitzi Drive Residential

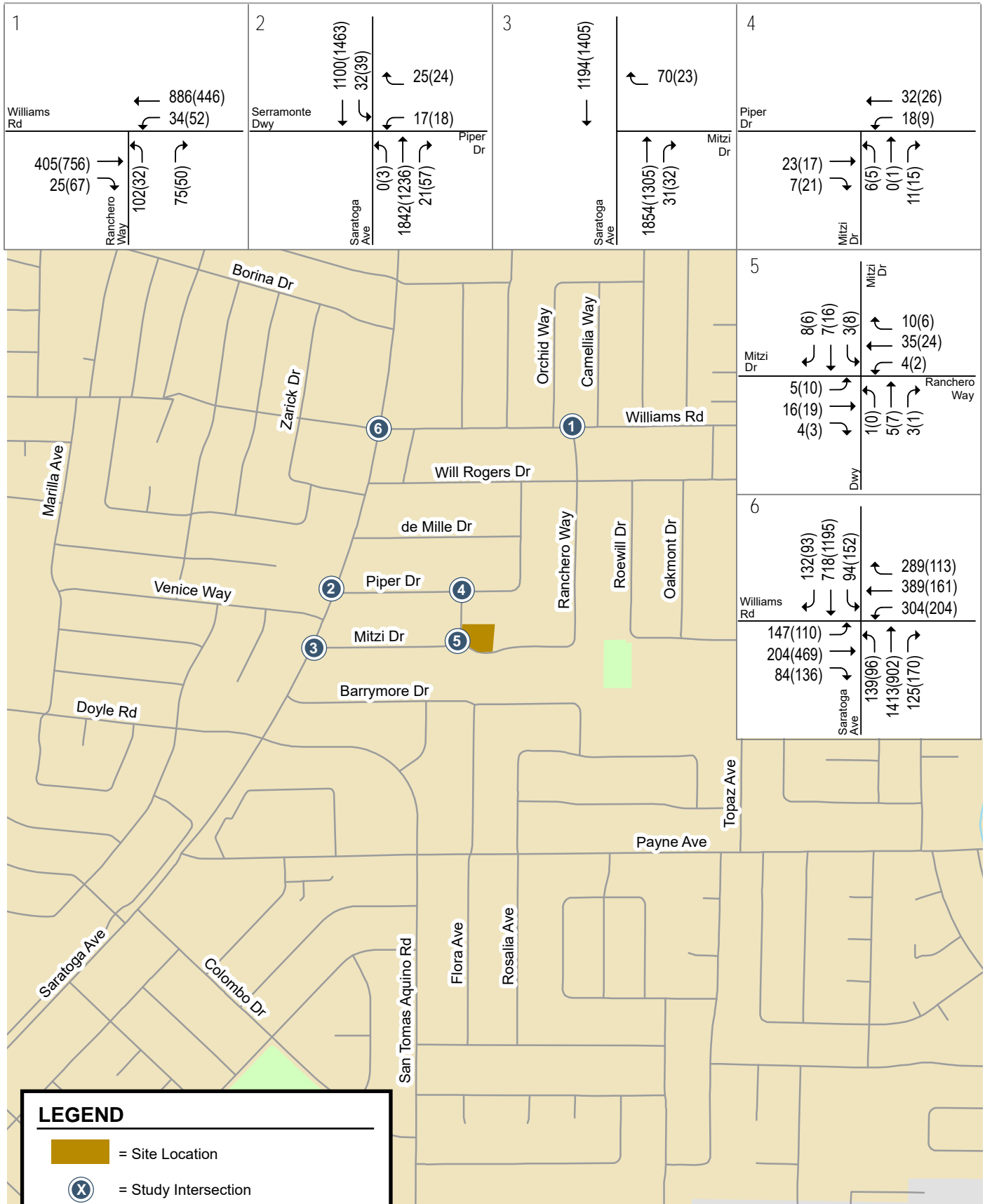


Figure 10
Background Traffic Volumes

Mitzi Drive Residential

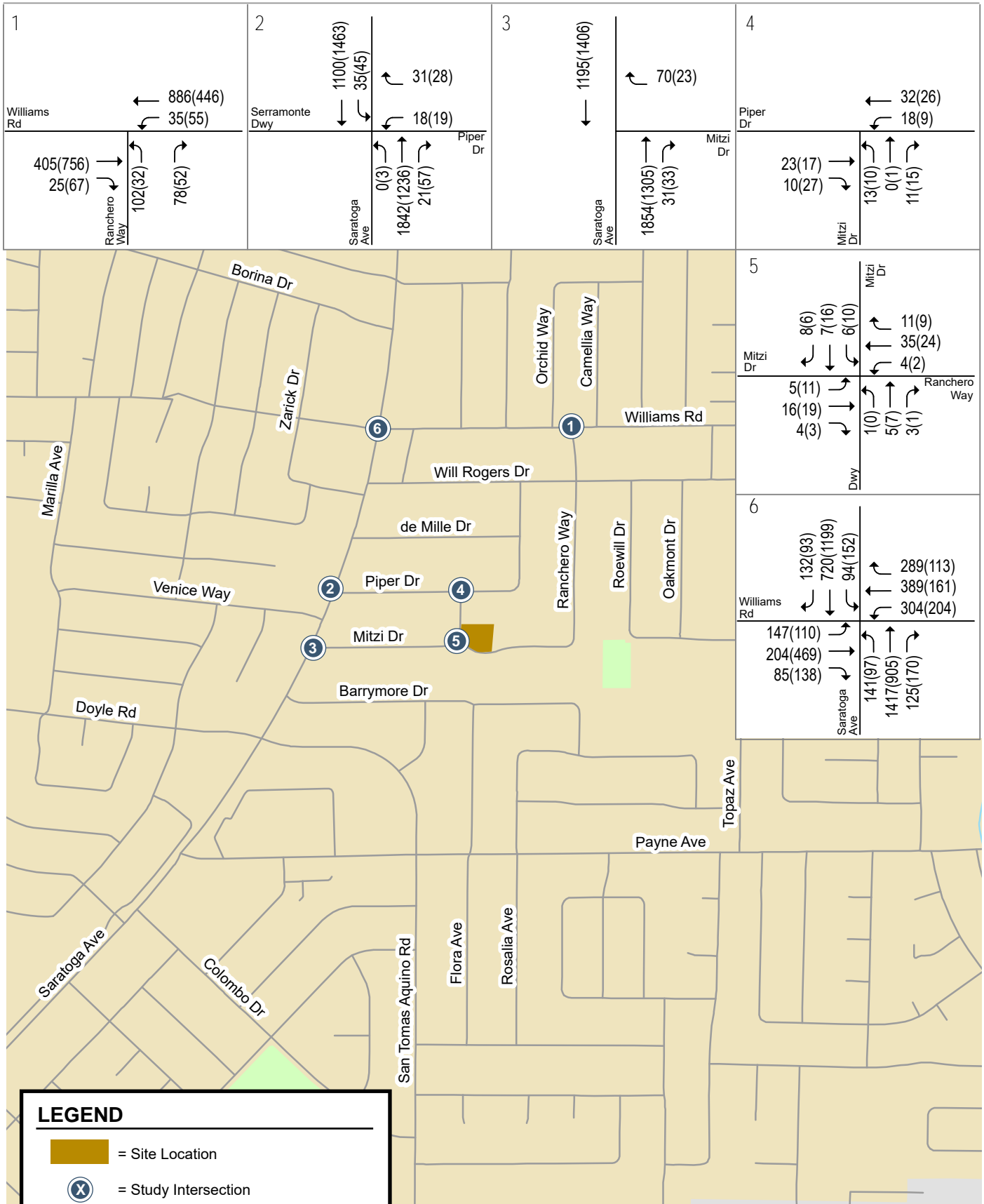


Figure 11
Background Plus Project Traffic Volumes

Mitzi Drive Residential

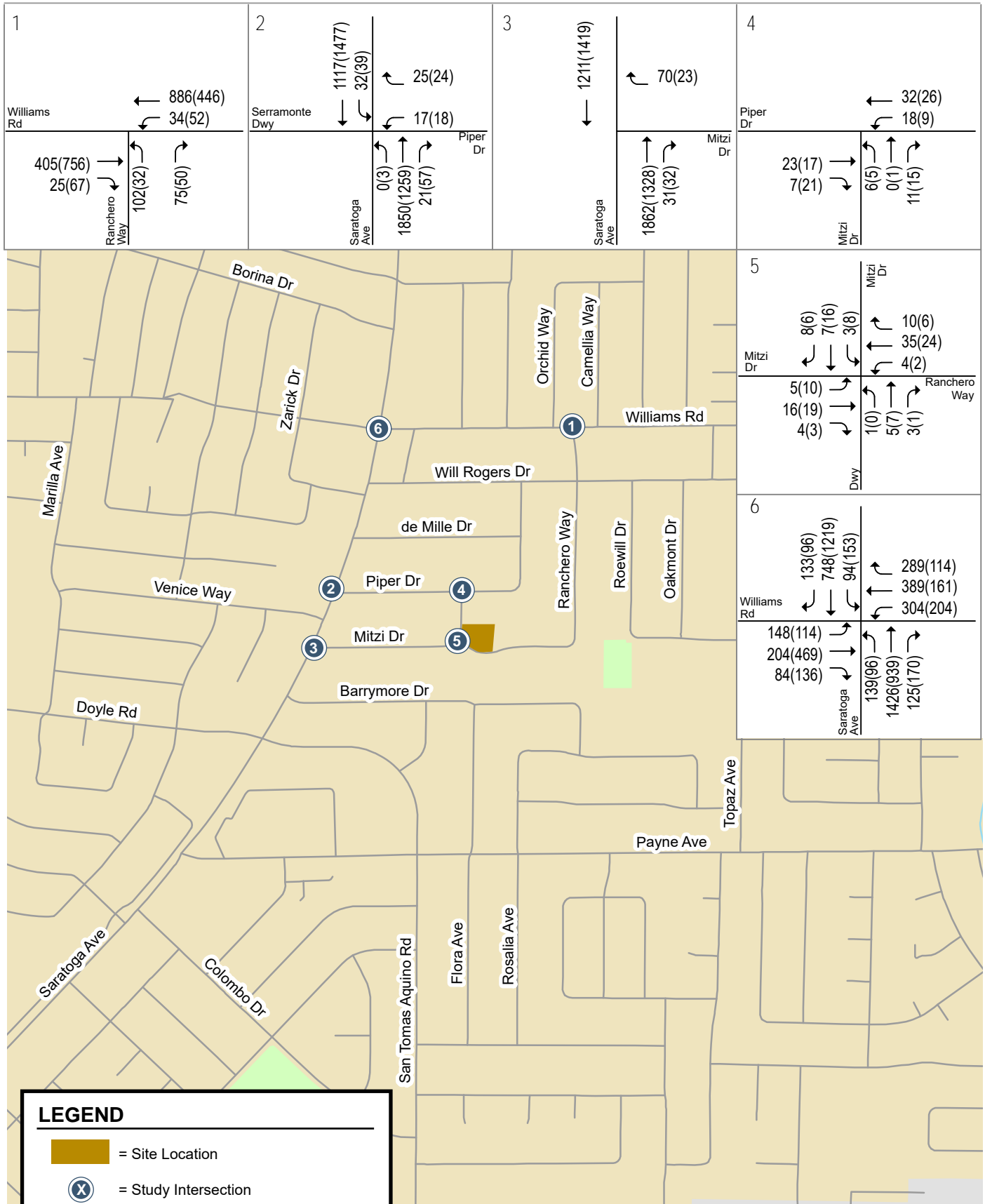


Figure 12
Cumulative Project Traffic Volumes

Intersection Traffic Operations under Background and Project Conditions

Intersection traffic operations at the study intersections were evaluated against the City of San Jose level of service standard (LOS D). The City of San Jose does not have a level of service standard for unsignalized intersections. The results of the intersection level of service analysis (see Table 5) show that the signalized intersection of Saratoga Avenue and Williams Road would operate at an acceptable level of service, LOS D or better, during the AM and PM peak hours under background and background plus project conditions.

The results of the intersection level of service analysis show that the left turn from Piper Drive to Saratoga Avenue operates at LOS F. Based on the traffic counts, there are only 17 vehicles that experience LOS F during the AM peak hour. Field observations showed a maximum queue of 2 vehicles making the left turn. It should be noted that vehicles can take an alternative route to travel southbound on Saratoga Avenue. Vehicles exiting the project site could use Ranchero Way to make a left turn onto Williams Road and then make a left turn at the signalized intersection of Saratoga Avenue and Williams Road.

Peak-Hour Signal Warrant Analysis

The Saratoga Avenue/Piper Drive intersection was evaluated on the basis of the Peak-Hour Volume Signal Warrant, (Warrant #3 – Part B) described in the *California Manual on Uniform Traffic Control Devices* (MUTCD), 2014 Edition. This method provides an indication whether peak-hour traffic volumes are, or would be, sufficient to justify installation of a traffic signal. Intersections that meet the peak hour warrant are subject to further analysis before determining that a traffic signal is necessary. Other options such as traffic control devices, signage, or geometric changes may be preferable based on existing field conditions.

The results of the signal warrant checks indicate that the AM and PM peak-hour volumes at the intersection would not meet the signal warrant under existing, background, background plus project, or cumulative conditions. The peak-hour signal warrant sheets are contained in Appendix D. Field observations showed that there were no difficulties for the left-turn traffic on Piper Drive to find gaps in traffic and make turns, and the vehicle queues and delay for the westbound traffic on Piper Drive were found to be acceptable.

The intersection level of service calculation sheets are included in Appendix C.

Table 5
Background Plus Project Intersection Levels of Service

| Study Number | Intersection | Peak Hour | Background Conditions | | | | | |
|--------------|--|-----------|--------------------------|----------|--------------------------|----------|-------------------------------|--------------------|
| | | | No Project | | With Project | | | |
| | | | Delay (sec) ¹ | LOS | Delay (sec) ¹ | LOS | Incr. in Critical Delay (sec) | Incr. In Crit. V/C |
| 1 | Ranchero Way and Williams Road (unsignalized) | AM | 23.6 | C | 23.7 | C | 0.1 | 0.001 |
| | | PM | 21.6 | C | 21.7 | C | 0.0 | 0.002 |
| 2 | Saratoga Avenue and Piper Drive (unsignalized) | AM | >90 | F | >90 | F | 0.2 | 0.038 |
| | | PM | 41.3 | E | 41.4 | E | 0.1 | 0.024 |
| 3 | Saratoga Avenue and Mitzi Drive (unsignalized) | AM | 15.1 | C | 15.1 | C | 0.0 | 0.000 |
| | | PM | 11.7 | B | 11.7 | B | 0.0 | 0.000 |
| 4 | Piper Drive and Mitzi Drive (unsignalized) | AM | 8.7 | A | 8.8 | A | 0.4 | 0.003 |
| | | PM | 8.6 | A | 8.7 | A | 0.2 | 0.000 |
| 5 | Ranchero Way and Mitzi Drive (unsignalized) | AM | 9.2 | A | 9.2 | A | 0.0 | 0.000 |
| | | PM | 9.2 | A | 9.3 | A | 0.0 | 0.000 |
| 6 | Saratoga Avenue and Williams Road | AM | 37.0 | D | 37.0 | D | 0.0 | 0.001 |
| | | PM | 40.5 | D | 40.5 | D | 0.1 | 0.002 |

Note:

¹ Delays based on worst approach delay for unsignalized intersections and average delay for signalized intersections.

Bold indicates a substandard level of service.

Intersection Traffic Operations under Cumulative Conditions

Intersection traffic operations at the study intersections were evaluated against the City of San Jose level of service standard (LOS D). The City of San Jose does not have a level of service standard for unsignalized intersections. The results of the intersection level of service analysis (see Table 6) show that the signalized intersection of Saratoga Avenue and Williams Road would operate at an acceptable level of service, LOS D or better, during the AM and PM peak hours under cumulative conditions.

The intersection level of service calculation sheets are included in Appendix C.

Table 6
Cumulative Intersection Levels of Service

| Study Number | Intersection | Peak Hour | Cumulative Conditions No Project | |
|--------------|--|-----------|-------------------------------------|----------|
| | | | Delay (sec) ¹ | LOS |
| 1 | Ranchero Way and Williams Road (unsignalized) | AM | 23.6 | C |
| | | PM | 21.6 | C |
| 2 | Saratoga Avenue and Piper Drive (unsignalized) | AM | >90 | F |
| | | PM | 43.3 | E |
| 3 | Saratoga Avenue and Mitzi Drive (unsignalized) | AM | 15.1 | C |
| | | PM | 11.8 | B |
| 4 | Piper Drive and Mitzi Drive (unsignalized) | AM | 8.7 | A |
| | | PM | 8.6 | A |
| 5 | Ranchero Way and Mitzi Drive (unsignalized) | AM | 9.2 | A |
| | | PM | 9.2 | A |
| 6 | Saratoga Avenue and Williams Road | AM | 36.9 | D |
| | | PM | 40.4 | D |

Note:
¹ Delays based on worst approach delay for unsignalized intersections and average delay for signalized intersections.
Bold indicates a substandard level of service.

Vehicle Queuing Analysis

Based on where the project would add trips, vehicle queuing was analyzed for four left-turn pockets at the Saratoga Avenue/Williams Road intersection, the Saratoga Avenue/Piper Drive intersection, and the Ranchero Way/Williams Road intersection (see Table 7). The queuing analysis indicates that the four left-turn pockets operate adequately during the AM and PM peak hours for existing, background, background plus project, and cumulative conditions.

Table 7
Intersection Vehicle Queuing Analysis Results

| Measurement | Saratoga Avenue and Williams Road | | Ranchero Way and Williams Road | | Saratoga Avenue and Piper Drive | | | |
|---------------------------------------|-----------------------------------|-----|--------------------------------|-----|---------------------------------|-----|------|------|
| | NBL | | WBL | | WBL | | SBL | |
| | AM | PM | AM | PM | AM | PM | AM | PM |
| Existing | | | | | | | | |
| Cycle/Delay ¹ (sec) | 120 | 130 | 8.3 | 9.7 | 93 | 93 | 17.1 | 12.1 |
| Volume (vphpl) | 138 | 96 | 34 | 52 | 17 | 18 | 32 | 39 |
| Total 95th % Queue (veh.) | 8 | 7 | 1 | 1 | 2 | 2 | 1 | 1 |
| Total 95th % Queue (ft.) ² | 200 | 175 | 25 | 25 | 50 | 50 | 25 | 25 |
| Total Storage | 300 | 300 | 75 | 75 | 50 | 50 | 200 | 200 |
| Adequate (Y/N) | Y | Y | Y | Y | Y | Y | Y | Y |
| Background | | | | | | | | |
| Cycle/Delay ¹ (sec) | 120 | 130 | 8.3 | 9.7 | 93 | 93 | 17.1 | 12.1 |
| Volume (vphpl) | 139 | 96 | 34 | 52 | 17 | 18 | 32 | 39 |
| Total 95th % Queue (veh.) | 8 | 7 | 1 | 1 | 2 | 2 | 1 | 1 |
| Total 95th % Queue (ft.) ² | 200 | 175 | 25 | 25 | 50 | 50 | 25 | 25 |
| Total Storage | 300 | 300 | 75 | 75 | 50 | 50 | 200 | 200 |
| Adequate (Y/N) | Y | Y | Y | Y | Y | Y | Y | Y |
| Background Plus Project | | | | | | | | |
| Cycle/Delay ¹ (sec) | 120 | 130 | 8.3 | 9.7 | 93 | 93 | 17.1 | 12.1 |
| Volume (vphpl) | 141 | 97 | 35 | 55 | 18 | 19 | 34 | 45 |
| Total 95th % Queue (veh.) | 9 | 7 | 1 | 1 | 2 | 2 | 1 | 1 |
| Total 95th % Queue (ft.) ² | 225 | 175 | 25 | 25 | 50 | 50 | 25 | 25 |
| Total Storage | 300 | 300 | 75 | 75 | 50 | 50 | 200 | 200 |
| Adequate (Y/N) | Y | Y | Y | Y | Y | Y | Y | Y |
| Cumulative | | | | | | | | |
| Cycle/Delay ¹ (sec) | 120 | 130 | 8.3 | 9.7 | 93 | 93 | 17.1 | 12.1 |
| Volume (vphpl) | 139 | 96 | 34 | 52 | 17 | 18 | 32 | 39 |
| Avg. Queue (veh./ln.) | 4.6 | 3.5 | 0.1 | 0.1 | 0.4 | 0.5 | 0.2 | 0.1 |
| Avg. Queue ² (ft./ln.) | 116 | 87 | 2 | 4 | 11 | 12 | 4 | 3 |
| 95th % Queue (veh./ln.) | 8 | 7 | 0 | 0 | 2 | 2 | 1 | 1 |
| 95th % Queue (ft./ln.) | 200 | 175 | 0 | 0 | 50 | 50 | 25 | 25 |
| Storage (ft./ln.) | 300 | 300 | 75 | 75 | 50 | 50 | 200 | 200 |
| Adequate (Y/N) | Y | Y | Y | Y | Y | Y | Y | Y |

Notes:
WBL = westbound left movement; NBL = northbound left movement; SBL = southbound left movement
¹ Vehicle queue calculations based on cycle length for signalized intersections and worst approach delay for unsignalized intersections.
² Assumes 25 Feet Per Vehicle Queued.

Vehicular Access and Circulation

The site access and circulation evaluation is based on the June 28, 2018 site plan prepared by Anderson Architects, Inc. (see Figure 13). Site access and on-site vehicular circulation were reviewed in accordance with generally accepted traffic engineering standards.

Site Access

The project generated traffic would access the site via a full-access driveway on Mitzi Drive that leads to the underground parking garage. As shown in Table 4, there would be 4 inbound and 10 outbound trips at the full-access driveway during the AM peak hour, and 10 inbound and 7 outbound trips during the PM peak hour. This modest volume of traffic would not face any delays in getting into and out of the garage.

The project proposes to adjust the property line back from the existing property line. This would significantly widen Rancho Way along its frontage to match to the adjacent curb line. This would enhance pedestrian circulation by providing a sidewalk and would allow room for on-street parking along Rancho Way, which Rancho Way currently lacks.

According to the City of San Jose Department of Transportation (DOT) Geometric Design Guidelines, the typical width for a two-way driveway for a residential (multi-family) building is 26 feet wide. The main two-way driveway on Mitzi Drive is shown to be approximately 21 feet wide, which does not meet City Standard. Thus, the driveway should be widened to 26 feet to meet the City requirement.

Sight Distance

The proposed project driveway should be free and clear of any obstructions to optimize sight distance. Providing the appropriate sight distance reduces the likelihood of a collision at the driveway and provides drivers with the ability to locate sufficient gaps in traffic and exit the site. There are no landscaping features shown on the site plan between Rancho Way and the driveway. It is recommended that landscaping near the driveway be kept below three feet to maintain sight distance. The main driveway leads to the underground parking lot that should remain clear of any obstructions for optimum sight distance. Vehicles using the driveway would have sufficient sight distance in both directions on Mitzi Drive and would be able to see vehicles on Rancho Way.

On-Site Circulation

On-site vehicular circulation was reviewed in accordance with generally accepted traffic engineering standards. The project would have a full-access driveway on Mitzi Drive that leads to an underground parking garage. In the underground parking garage, there would be a drive aisle that leads to the parking spaces and the stacked parking spaces. The perimeter drive aisle would have 90-degree perpendicular parking spaces. The drive aisle width (26 feet) would provide sufficient space for vehicles to back out of the of the parking stalls. Generally, the proposed plan would provide vehicle traffic with adequate connectivity through the parking areas. However, the site plan shows one fairly long dead-end parking aisle. Generally, dead-end aisles are undesirable because vehicles finding all parking spaces occupied would need to back out. The site plan does not show sufficient space to turn around at the end of the dead-end aisle. Since the stalls on the dead-end aisle are likely to be used by guests, turnaround space should be provided. This could be accomplished by removing one of the parking stalls.

The project proposes to install two stacked parking systems in the underground parking garage. The stacked parking system is using the Klaus 4100 model and 4300 model stackers. The stackers are proposed to hold 18 and 23 parking spaces, respectively. According to the manufacturer's website, the dimensions of the sliding door are approximately 2500 mm wide by 2000 mm high (8.2 ft by 6.6 ft). The means that the stacked parking system can accommodate most vehicles except large SUVs and trucks.

Truck Access and Circulation

Loading

The project is not proposing any off-street loading spaces. According to the City of San Jose parking standards (San Jose Municipal Code Chapter 20.70), multifamily residential units greater than 50 units are required to provide at least one off-street loading space. Since the project is proposing 46 multifamily units, the project is not required to provide an off-street loading space. On-street parking is allowed on the adjacent streets that could be used for moving vehicles.

Garbage Collection

The site plan shows the trash room to be located on the ground floor near the southeast corner of the project site. Garbage collection activities for the project are not expected to occur on site due to access limitations. Thus, the trash bins should be moved to the curb along Rancho Way on designated garbage collection days. The trash bins also should be removed from the public right-of-way immediately after garbage pickup as to not impact pedestrian or traffic operations.

Emergency Vehicle Access

Emergency vehicle (EVA) access would be provided along Rancho Way and Mitzi Lane and at the project driveway. The City of San Jose Fire Code requires driveways to provide at least 20 feet for fire access. The project driveway would measure approximately 21 feet wide, and therefore would comply with the City's fire code.

Pedestrian, Bicycle and Transit Facilities

All new development projects in San Jose should encourage multi-modal travel, consistent with the goals of the City's General Plan. It is the goal of the General Plan that all development projects accommodate and encourage the use of non-automobile transportation modes to achieve San Jose's mobility goals and reduce vehicle trip generation and vehicle miles traveled. In addition, the adopted City Bike Master Plan establishes goals, policies and actions to make bicycling a daily part of life in San Jose. The Master Plan includes designated bike lanes along all City streets, as well as on designated bike corridors. In order to further the goals of the City, pedestrian and bicycle facilities should be encouraged with new development projects.

Pedestrian Access and Circulation

The existing network of sidewalks and crosswalks in the immediate vicinity of the project site has good connectivity and provides pedestrians with safe routes to various points of interest in the study area, including nearby bus stops on Saratoga Avenue and Williams Road. The project would provide sidewalks along its frontage, which now are lacking. The closest school to the site is Anderson Elementary School, which is located west of the project site. However, the walking distance to the school is more than ½ mile, so it is unlikely that students would walk to school.

Bicycle Access and Circulation

There are bike lanes on Williams Road. The local streets of Mitzi Drive, Rancho Way, and Piper Drive carry low traffic volume and are conducive to bicyclists. The project would provide on-site bicycle parking. The existing bike lanes provide bicyclists with safe routes to various points of interest in the study area, including nearby bus stops on Saratoga Avenue and Williams Road and Anderson Elementary School. The elementary school is just over ½ mile from the project site, and bicyclists could ride there using bike lanes and low-volume residential streets.

Transit Service

The site is well-served by bus transit. There are three VTA local bus lines (Route 25, 57 and 58) that serve the immediate project area. The bus stops closest to the project site are on Saratoga Avenue at Mitzi Drive. The bus routes run throughout the day and on weekends with 15-30 minute headways.

Neighborhood Interface

The project site is situated adjacent to an existing residential neighborhood. The effect the project would have on local streets was evaluated for Mitzi Drive and Rancho Way.

The average daily traffic (ADT) volumes on three roadway segments and the estimated daily trips added to these roadway segments as a result of the project are shown below in Table 8. Since the City of San Jose has no established standard or significance threshold regarding daily traffic on neighborhood streets, the roadway volume data are presented here for informational purposes.

Table 8
Average Daily Traffic on Surrounding Streets

| Roadway Segment | Existing Weekday ADT Counts ¹ | Daily Project Trips | Existing + Project Volumes | % Change |
|--|--|---------------------|----------------------------|----------|
| Mitzi Drive b/t Saratoga Avenue and Rancho Way | 829 | 10 | 839 | 1.2% |
| Rancho Way b/t Mitzi Drive and Will Rogers Drive | 873 | 52 | 925 | 6.0% |
| Mitzi Drive b/t Piper Drive and Rancho Way | 501 | 114 | 615 | 22.8% |

Notes:
1. 24-hour tube counts were conducted November 6, 2018.

Based on the assignment of project generated trips, the project is expected to add 10 daily trips to Mitzi Drive east of Saratoga Avenue, 52 daily trips to Rancho Way east of Mitzi Drive, and 114 daily trips to Mitzi Drive north of Rancho Way.

Construction Activities

Typical activities related to the construction of any development could include lane narrowing and/or lane closures and sidewalk closures. Since the site has no sidewalks, it is unlikely that construction would affect pedestrian facilities. In the event of any type of street closure, clear signage (e.g., closure and detour signs) must be provided to ensure vehicles and bicyclists are able to adequately reach their

intended destinations safely. The project would be required to submit a construction management plan for City approval that addresses schedule, closures/detours, staging, parking, and truck routes.

Parking

Parking provided on the site was evaluated based on the City of San Jose parking standards (San Jose Municipal Code Chapter 20.90, Table 20-190). The vehicle parking requirement for multifamily residential use is 1.25 spaces per studio and 1-bedroom unit, inclusive of guest spaces. The project proposes 46 units. Therefore, the project is required to provide a minimum of 58 on-site parking spaces. The project proposes 61 on-site parking spaces, which exceeds the City's parking requirement. As described earlier, the garage includes a dead-end aisle that should provide turn-around space. That would result in the loss of one space. The project still would meet the City's parking requirement. 41 of the parking spaces would be supplied in mechanical parking stackers, and there would be 19 regular spaces. Guests should be assigned to the regular spaces because it is unlikely that guests would be familiar with the operation of the stackers.

The bicycle parking requirement for a multifamily housing use is one space per 4 units. The project proposes 46 units, so the project is required to provide 12 bicycle parking spaces. The site plan shows 12 bicycle parking spaces in the underground parking garage and 2 bicycle parking spaces on the ground floor, which would meet the City's bicycle parking requirement. Also, the steep garage ramp would be difficult for bicycles, and the use of the elevator for bicycles would be awkward. The bicycle parking should be relocated to the ground floor.

The motorcycle parking requirement for a multifamily housing use is one space per 4 units. The project proposes 46 units, so the project is required to provide 12 motorcycle parking spaces. The site plan shows 12 motorcycle parking spaces in the underground parking garage, which would meet the City's motorcycle parking requirement.

5. Conclusions

This study was conducted for the purpose of identifying the potential transportation impacts related to the proposed development. The transportation impacts of the project were evaluated following the standards and methodologies established in the City of San Jose's *Transportation Analysis Handbook*. Based on the City of San Jose's Transportation Analysis Policy and *Transportation Analysis Handbook*, the TA report for the project includes a CEQA transportation analysis and a local transportation analysis (LTA).

CEQA Transportation Impacts

Project Vehicle Miles Traveled (VMT) Analysis

Because the VMT generated by the project (8.52 per resident) is less than the threshold of 10.12 VMT per capita, the project would not result in a significant transportation impact on VMT, and mitigation measures are not required to reduce the VMT.

CEQA Cumulative Impacts

The project is consistent with the General Plan goals and policies for the following reasons:

- The project site is near bus stops and bicycle lanes on Williams Road.
- The project would increase the residential density in the project area.
- The project would provide bicycle parking.

Therefore, the project would be considered as part of the cumulative solution to meet the General Plan's long-range transportation goals and would result in a less-than-significant cumulative impact.

Local Transportation Effects

Intersection Traffic Operations

The results of the analysis show that the Saratoga Avenue and Williams Road intersection operates at an acceptable level of service (LOS D or better) during the AM and PM peak hour.

The results of the analysis show that the left turn from Piper Drive to Saratoga Avenue operates at LOS F during the AM peak hour. However, the City of San Jose does not have a level of service standard for unsignalized intersections. Motorists have an alternative route to travel to southbound Saratoga Avenue. Vehicles exiting the project site can use Ranchero Way to make a left turn onto Williams Road

and then make a left turn at the signalized intersection of Saratoga Avenue and Williams Road. Field observations showed a vehicle queue of 2 vehicles making the left turn from Piper Drive to Saratoga Avenue. The results of the signal warrant analysis indicate that the AM and PM peak-hour volumes at the intersection would not meet the signal warrant.

Other Transportation Issues

The site plan shows adequate site access and on-site circulation, and no significant operational issues are expected to occur as a result of the project. The project would enhance pedestrian circulation by building sidewalks along its frontage where none exist today. The existing transit and bicycle facilities in the study area are sufficient to serve the project.

Hexagon has the following recommendations resulting from the parking, site access, and circulation analysis.

- The project site plan should be revised to meet the City's minimum requirement of 26 feet for a two-way driveway.
- Since the stalls on the dead-end aisle are likely to be used by guests, turnaround space should be provided. This could be accomplished by removing one of the parking stalls.
- The bicycle parking should be moved to the ground floor.
- Landscaping near the driveway should be kept below three feet to maintain sight distance.

4146 Mitzi Drive

Technical Appendices

November 29, 2018

Appendix A

San Jose VMT Evaluation Tool Summary Report

CITY OF SAN JOSE VEHICLE MILES TRAVELED EVALUATION TOOL SUMMARY REPORT

PROJECT:

| | |
|----------------------------|--|
| Name: Mitzi Apartments | Tool Version: 3/14/2018 |
| Location: 4146 Mitzi Drive | Date: 11/6/2018 |
| Parcel: 29916001 | Parcel Type: Suburb with Multifamily Housing |
| Proposed Parking: | Vehicles: 61 Bicycles: 12 |

LAND USE:

| | | |
|----------------------------------|---|----------------|
| Residential: | Percent of All Residential Units | |
| Single Family 0 DU | Extremely Low Income (≤ 30% MFI) | 0 % Affordable |
| Multi Family 46 DU | Very Low Income (> 30% MFI, ≤ 50% MFI) | 0 % Affordable |
| Subtotal 46 DU | Low Income (> 50% MFI, ≤ 80% MFI) | 0 % Affordable |
| Office: 0 KSF | | |
| Retail: 0 KSF | | |
| Industrial: 0 KSF | | |

VMT REDUCTION STRATEGIES

Tier 1 - Project Characteristics

| | |
|--|------|
| Increase Residential Density | |
| Existing Density (DU/Residential Acres in half-mile buffer) | 9 |
| With Project Density (DU/Residential Acres in half-mile buffer) | 9 |
| Increase Development Diversity | |
| Existing Activity Mix Index | 0.30 |
| With Project Activity Mix Index | 0.30 |
| Integrate Affordable and Below Market Rate | |
| Extremely Low Income BMR units | 0 % |
| Very Low Income BMR units | 0 % |
| Low Income BMR units | 0 % |
| Increase Employment Density | |
| Existing Density (Jobs/Commercial Acres in half-mile buffer) | 11 |
| With Project Density (Jobs/Commercial Acres in half-mile buffer) | 11 |

Tier 2 - Multimodal Infrastructure

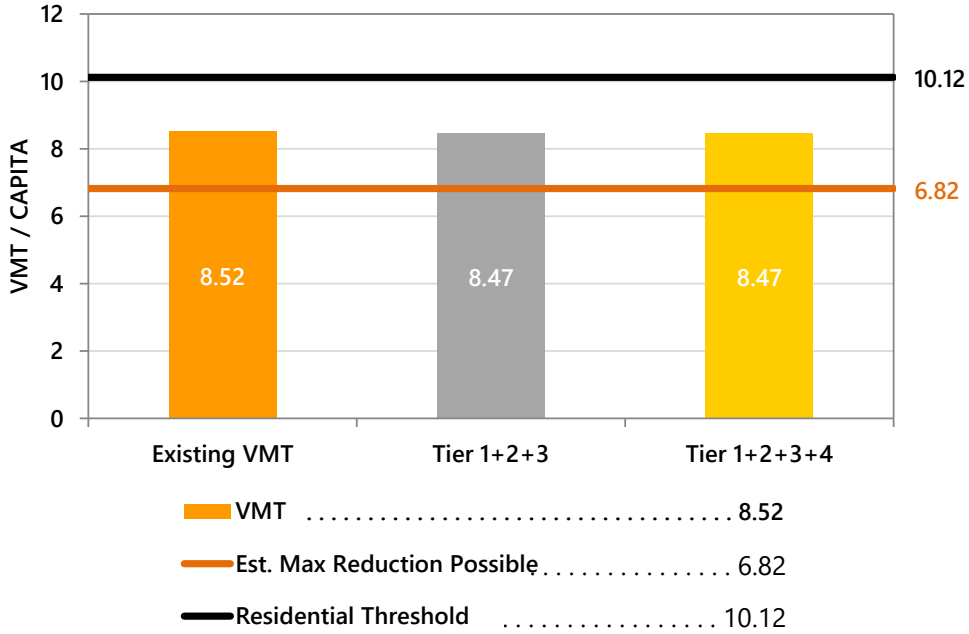
Tier 3 - Parking

Tier 4 - TDM Programs

CITY OF SAN JOSE VEHICLE MILES TRAVELED EVALUATION TOOL SUMMARY REPORT

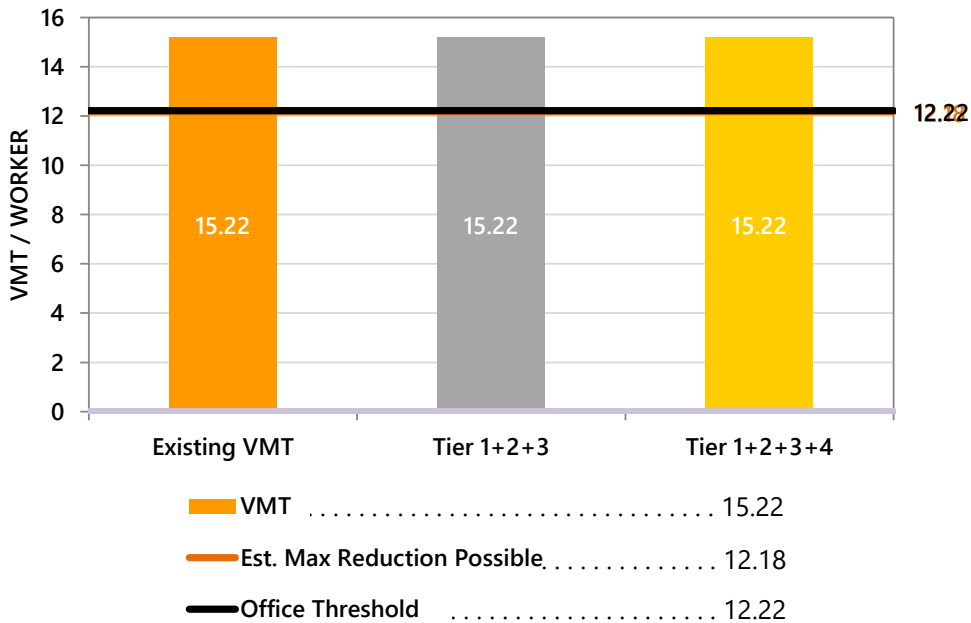
RESIDENTIAL ONLY

The tool estimates that the project would generate per capita VMT below the City's threshold.



EMPLOYMENT ONLY

The tool estimates that the project would generate per non-industrial worker VMT above the City's threshold.



Appendix B

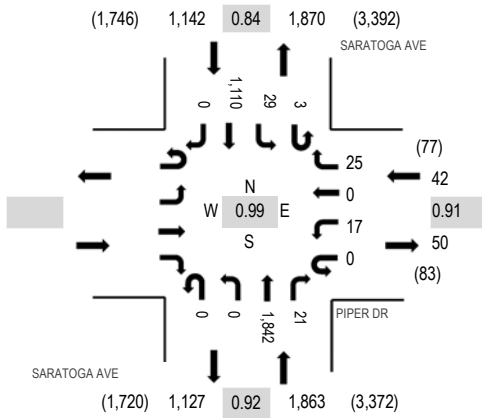
Traffic Counts



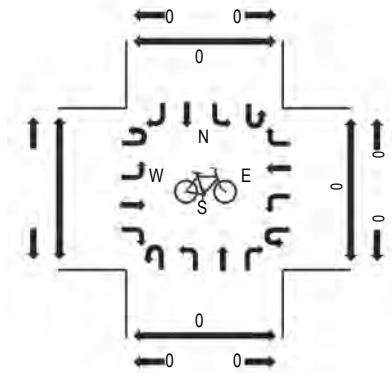
(303) 216-2439
www.alltrafficdata.net

Location: 2 SARATOGA AVE & PIPER DR AM
Date: Tuesday, November 6, 2018
Peak Hour: 07:30 AM - 08:30 AM
Peak 15-Minutes: 07:30 AM - 07:45 AM

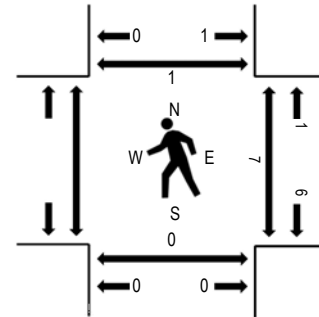
Peak Hour - All Vehicles



Peak Hour - Bicycles



Peak Hour - Pedestrians



Note: Total study counts contained in parentheses.

Traffic Counts

| Interval Start Time | Eastbound | | | | PIPER DR Westbound | | | | SARATOGA AVE Northbound | | | | SARATOGA AVE Southbound | | | | Total | Rolling Hour | Pedestrian Crossings | | | |
|---------------------|-----------|------|------|-------|--------------------|------|------|-------|-------------------------|------|------|-------|-------------------------|------|------|-------|-------|--------------|----------------------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | | | West | East | South | North |
| 7:00 AM | | | | | 0 | 4 | 0 | 9 | 0 | 0 | 281 | 2 | 0 | 7 | 124 | 0 | 427 | 2,403 | 0 | 0 | 0 | |
| 7:15 AM | | | | | 0 | 1 | 0 | 10 | 0 | 0 | 320 | 4 | 0 | 4 | 116 | 0 | 455 | 2,738 | 1 | 0 | 0 | |
| 7:30 AM | | | | | 0 | 4 | 0 | 10 | 0 | 0 | 475 | 2 | 0 | 8 | 267 | 0 | 766 | 3,047 | 3 | 0 | 1 | |
| 7:45 AM | | | | | 0 | 8 | 0 | 5 | 0 | 0 | 443 | 6 | 2 | 7 | 284 | 0 | 755 | 2,969 | 3 | 0 | 0 | |
| 8:00 AM | | | | | 0 | 1 | 0 | 4 | 0 | 0 | 413 | 6 | 0 | 11 | 327 | 0 | 762 | 2,792 | 1 | 0 | 0 | |
| 8:15 AM | | | | | 0 | 4 | 0 | 6 | 0 | 0 | 511 | 7 | 1 | 3 | 232 | 0 | 764 | | 0 | 0 | 0 | |
| 8:30 AM | | | | | 0 | 2 | 0 | 5 | 0 | 0 | 505 | 5 | 2 | 1 | 168 | 0 | 688 | | 1 | 0 | 0 | |
| 8:45 AM | | | | | 0 | 2 | 0 | 2 | 0 | 0 | 388 | 4 | 0 | 6 | 176 | 0 | 578 | | 4 | 0 | 0 | |

Peak Rolling Hour Flow Rates

| Vehicle Type | Eastbound | | | | Westbound | | | | Northbound | | | | Southbound | | | | Total |
|--------------------|-----------|------|------|-------|-----------|------|------|-------|------------|------|-------|-------|------------|------|-------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | |
| Articulated Trucks | | | | | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 1 | 9 | 0 | 18 |
| Lights | | | | | 0 | 16 | 0 | 23 | 0 | 0 | 1,810 | 21 | 3 | 27 | 1,075 | 0 | 2,975 |
| Mediums | | | | | 0 | 1 | 0 | 2 | 0 | 0 | 24 | 0 | 0 | 1 | 26 | 0 | 54 |
| Total | | | | | 0 | 17 | 0 | 25 | 0 | 0 | 1,842 | 21 | 3 | 29 | 1,110 | 0 | 3,047 |



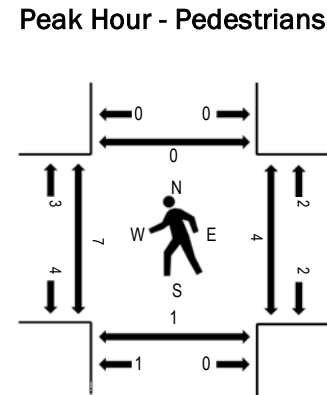
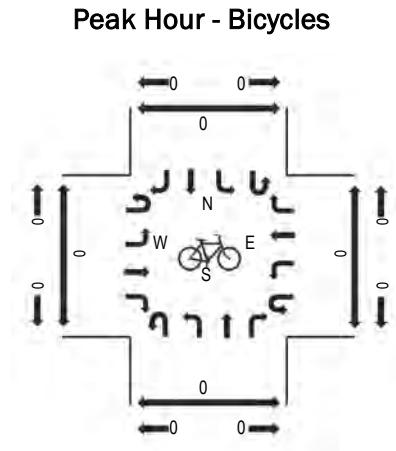
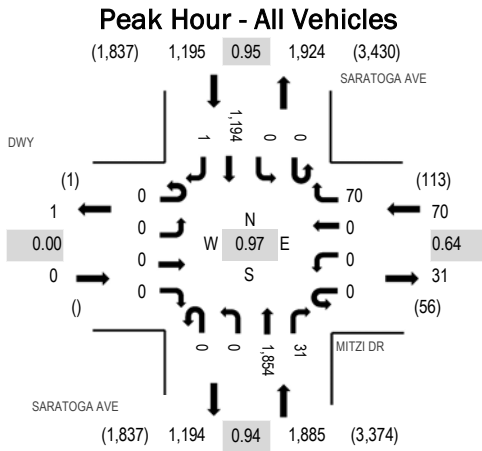
(303) 216-2439
www.alltrafficdata.net

Location: 3 SARATOGA AVE & MITZI DR AM

Date: Tuesday, November 6, 2018

Peak Hour: 07:30 AM - 08:30 AM

Peak 15-Minutes: 08:15 AM - 08:30 AM



Note: Total study counts contained in parentheses.

Traffic Counts

| Interval Start Time | DWY Eastbound | | | | MITZI DR Westbound | | | | SARATOGA AVE Northbound | | | | SARATOGA AVE Southbound | | | | Total | Rolling Hour | Pedestrian Crossings | | | |
|---------------------|---------------|------|------|-------|--------------------|------|------|-------|-------------------------|------|------|-------|-------------------------|------|------|-------|-------|--------------|----------------------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | | | West | East | South | North |
| 7:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 0 | 276 | 9 | 0 | 0 | 137 | 0 | 437 | 2,463 | 1 | 1 | 1 | 0 |
| 7:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 317 | 10 | 0 | 0 | 135 | 0 | 472 | 2,814 | 5 | 2 | 1 | 0 |
| 7:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 28 | 0 | 0 | 437 | 7 | 0 | 0 | 288 | 0 | 760 | 3,150 | 1 | 2 | 1 | 0 |
| 7:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 18 | 0 | 0 | 456 | 7 | 0 | 0 | 313 | 0 | 794 | 3,080 | 4 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 16 | 0 | 0 | 449 | 10 | 0 | 0 | 312 | 1 | 788 | 2,861 | 2 | 2 | 0 | 0 |
| 8:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 512 | 7 | 0 | 0 | 281 | 0 | 808 | | 0 | 0 | 0 | 0 |
| 8:30 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 10 | 0 | 0 | 499 | 2 | 0 | 0 | 178 | 0 | 690 | | 0 | 2 | 0 | 0 |
| 8:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 372 | 4 | 0 | 0 | 192 | 0 | 575 | | 2 | 3 | 1 | 0 |

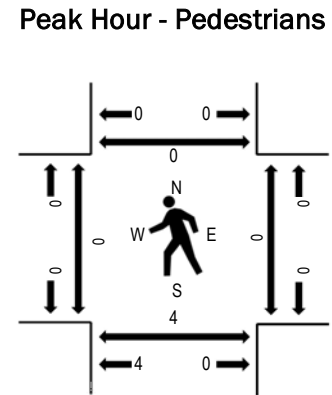
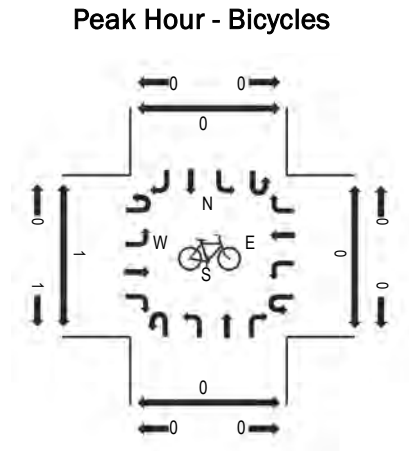
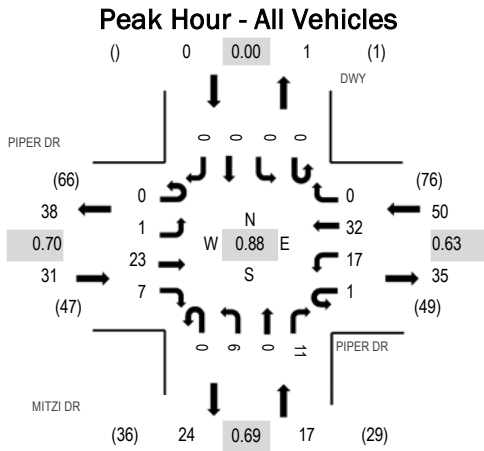
Peak Rolling Hour Flow Rates

| Vehicle Type | Eastbound | | | | Westbound | | | | Northbound | | | | Southbound | | | | Total |
|--------------------|-----------|------|------|-------|-----------|------|------|-------|------------|------|-------|-------|------------|------|-------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 10 | 0 | 18 |
| Lights | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 67 | 0 | 0 | 1,821 | 31 | 0 | 0 | 1,162 | 1 | 3,082 |
| Mediums | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 25 | 0 | 0 | 0 | 22 | 0 | 50 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 70 | 0 | 0 | 1,854 | 31 | 0 | 0 | 1,194 | 1 | 3,150 |



(303) 216-2439
www.alltrafficdata.net

Location: 4 MITZI DR & PIPER DR AM
Date: Tuesday, November 6, 2018
Peak Hour: 07:30 AM - 08:30 AM
Peak 15-Minutes: 07:30 AM - 07:45 AM



Note: Total study counts contained in parentheses.

Traffic Counts

| Interval Start Time | PIPER DR Eastbound | | | | PIPER DR Westbound | | | | MITZI DR Northbound | | | | DWY Southbound | | | | Total | Rolling Hour | Pedestrian Crossings | | | |
|---------------------|--------------------|------|------|-------|--------------------|------|------|-------|---------------------|------|------|-------|----------------|------|------|-------|-------|--------------|----------------------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | | | West | East | South | North |
| 7:00 AM | 0 | 0 | 2 | 4 | 0 | 1 | 4 | 0 | 0 | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 14 | 89 | 0 | 0 | 1 | 0 |
| 7:15 AM | 0 | 0 | 1 | 1 | 0 | 1 | 9 | 0 | 0 | 5 | 0 | 3 | 0 | 0 | 0 | 0 | 20 | 96 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 5 | 0 | 0 | 7 | 13 | 0 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 28 | 98 | 0 | 0 | 1 | 0 |
| 7:45 AM | 0 | 0 | 9 | 2 | 0 | 5 | 5 | 0 | 0 | 2 | 0 | 4 | 0 | 0 | 0 | 0 | 27 | 81 | 0 | 0 | 1 | 0 |
| 8:00 AM | 0 | 0 | 5 | 3 | 1 | 2 | 5 | 0 | 0 | 2 | 0 | 3 | 0 | 0 | 0 | 0 | 21 | 63 | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 1 | 4 | 2 | 0 | 3 | 9 | 0 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 22 | | 0 | 0 | 2 | 0 |
| 8:30 AM | 0 | 0 | 2 | 2 | 0 | 2 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | | 0 | 0 | 2 | 2 |
| 8:45 AM | 0 | 0 | 4 | 0 | 0 | 1 | 3 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 9 | | 0 | 0 | 0 | 1 |

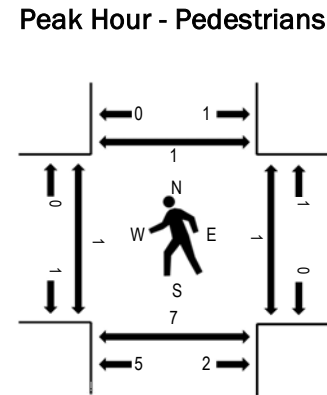
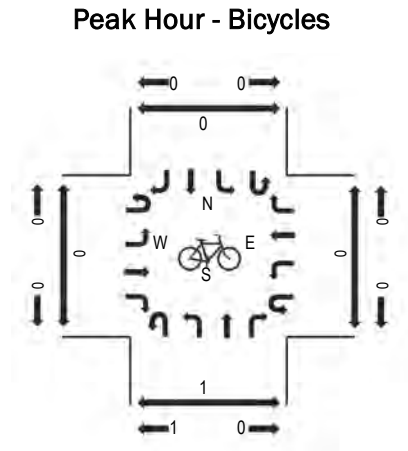
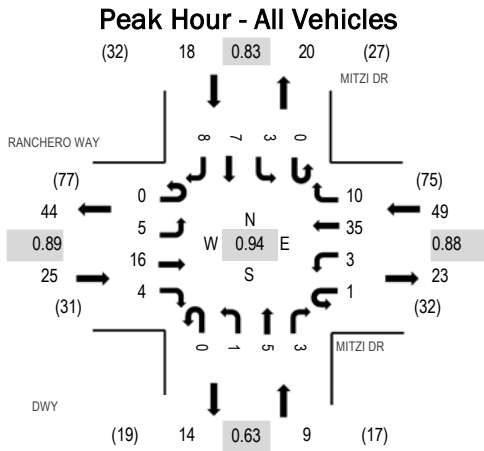
Peak Rolling Hour Flow Rates

| Vehicle Type | Eastbound | | | | Westbound | | | | Northbound | | | | Southbound | | | | Total | |
|--------------------|-----------|------|------|-------|-----------|------|------|-------|------------|------|------|-------|------------|------|------|-------|-------|---|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | | |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lights | 0 | 1 | 22 | 6 | 1 | 17 | 31 | 0 | 0 | 6 | 0 | 10 | 0 | 0 | 0 | 0 | 94 | |
| Mediums | 0 | 0 | 1 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 4 | |
| Total | 0 | 1 | 23 | 7 | 1 | 17 | 32 | 0 | 0 | 6 | 0 | 11 | 0 | 0 | 0 | 0 | 98 | |



(303) 216-2439
www.alltrafficdata.net

Location: 5 DWY & MITZI DR AM
Date: Tuesday, November 6, 2018
Peak Hour: 07:15 AM - 08:15 AM
Peak 15-Minutes: 07:30 AM - 07:45 AM



Note: Total study counts contained in parentheses.

Traffic Counts

| Interval Start Time | RANCHERO WAY Eastbound | | | | MITZI DR Westbound | | | | DWY Northbound | | | | MITZI DR Southbound | | | | Total | Rolling Hour | Pedestrian Crossings | | | |
|---------------------|------------------------|------|------|-------|--------------------|------|------|-------|----------------|------|------|-------|---------------------|------|------|-------|-------|--------------|----------------------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | | | West | East | South | North |
| 7:00 AM | 0 | 0 | 1 | 0 | 0 | 0 | 6 | 2 | 0 | 2 | 1 | 0 | 0 | 1 | 1 | 3 | 17 | 92 | 0 | 2 | 1 | 1 |
| 7:15 AM | 0 | 1 | 3 | 1 | 0 | 0 | 8 | 5 | 0 | 0 | 2 | 0 | 0 | 2 | 2 | 0 | 24 | 101 | 0 | 1 | 4 | 0 |
| 7:30 AM | 0 | 1 | 3 | 2 | 1 | 2 | 9 | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 5 | 27 | 92 | 1 | 0 | 0 | 1 |
| 7:45 AM | 0 | 2 | 5 | 0 | 0 | 0 | 9 | 1 | 0 | 0 | 1 | 1 | 0 | 1 | 2 | 2 | 24 | 78 | 0 | 0 | 2 | 0 |
| 8:00 AM | 0 | 1 | 5 | 1 | 0 | 1 | 9 | 2 | 0 | 1 | 2 | 1 | 0 | 0 | 2 | 1 | 26 | 63 | 0 | 0 | 1 | 0 |
| 8:15 AM | 0 | 0 | 2 | 0 | 0 | 1 | 6 | 0 | 0 | 0 | 1 | 1 | 0 | 1 | 2 | 1 | 15 | | 0 | 0 | 2 | 0 |
| 8:30 AM | 0 | 0 | 2 | 0 | 0 | 0 | 5 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 1 | 2 | 13 | | 1 | 0 | 4 | 0 |
| 8:45 AM | 0 | 1 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 1 | 9 | | 1 | 0 | 2 | 0 |

Peak Rolling Hour Flow Rates

| Vehicle Type | Eastbound | | | | Westbound | | | | Northbound | | | | Southbound | | | | Total |
|--------------------|-----------|------|------|-------|-----------|------|------|-------|------------|------|------|-------|------------|------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lights | 0 | 5 | 16 | 4 | 1 | 3 | 33 | 9 | 0 | 1 | 5 | 3 | 0 | 3 | 7 | 6 | 96 |
| Mediums | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 5 |
| Total | 0 | 5 | 16 | 4 | 1 | 3 | 35 | 10 | 0 | 1 | 5 | 3 | 0 | 3 | 7 | 8 | 101 |



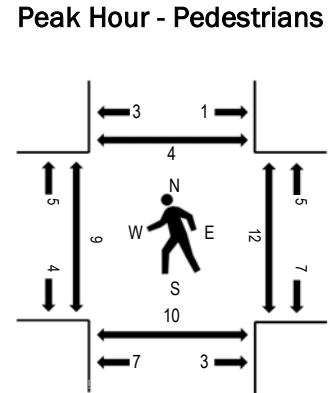
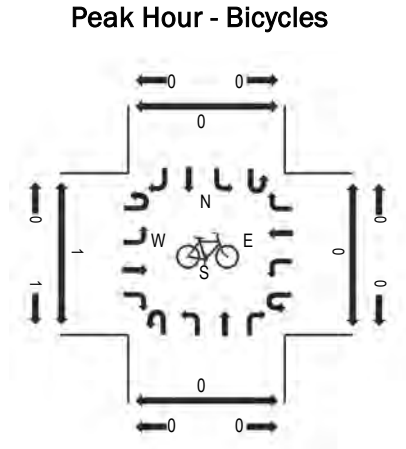
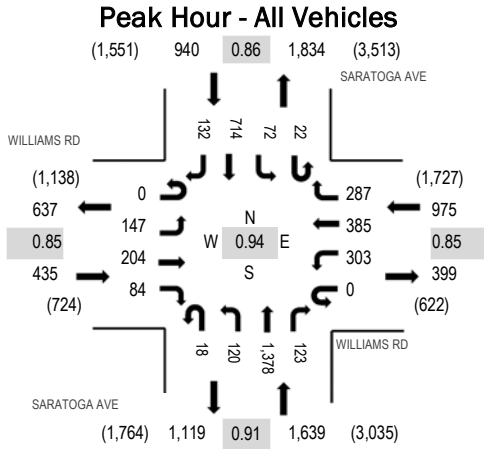
(303) 216-2439
www.alltrafficdata.net

Location: 6 SARATOGA AVE & WILLIAMS RD AM

Date: Tuesday, November 6, 2018

Peak Hour: 07:30 AM - 08:30 AM

Peak 15-Minutes: 08:00 AM - 08:15 AM



Note: Total study counts contained in parentheses.

Traffic Counts

| Interval Start Time | WILLIAMS RD Eastbound | | | | WILLIAMS RD Westbound | | | | SARATOGA AVE Northbound | | | | SARATOGA AVE Southbound | | | | Total | Rolling Hour | Pedestrian Crossings | | | |
|---------------------|-----------------------|------|------|-------|-----------------------|------|------|-------|-------------------------|------|------|-------|-------------------------|------|------|-------|-------|--------------|----------------------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | | | West | East | South | North |
| 7:00 AM | 0 | 27 | 14 | 12 | 0 | 34 | 71 | 80 | 5 | 9 | 246 | 12 | 5 | 14 | 89 | 8 | 626 | 3,309 | 2 | 1 | 4 | 2 |
| 7:15 AM | 0 | 34 | 28 | 8 | 0 | 41 | 111 | 83 | 1 | 20 | 267 | 11 | 14 | 11 | 84 | 26 | 739 | 3,742 | 3 | 0 | 4 | 0 |
| 7:30 AM | 0 | 31 | 30 | 21 | 0 | 86 | 116 | 103 | 3 | 31 | 313 | 25 | 7 | 8 | 140 | 32 | 946 | 3,989 | 2 | 3 | 1 | 2 |
| 7:45 AM | 0 | 31 | 61 | 19 | 0 | 76 | 102 | 57 | 3 | 33 | 296 | 49 | 4 | 20 | 196 | 51 | 998 | 3,969 | 3 | 5 | 4 | 0 |
| 8:00 AM | 0 | 38 | 53 | 21 | 0 | 100 | 103 | 65 | 8 | 30 | 344 | 24 | 7 | 21 | 213 | 32 | 1,059 | 3,728 | 2 | 1 | 3 | 0 |
| 8:15 AM | 0 | 47 | 60 | 23 | 0 | 41 | 64 | 62 | 4 | 26 | 425 | 25 | 4 | 23 | 165 | 17 | 986 | | 2 | 3 | 2 | 2 |
| 8:30 AM | 0 | 38 | 35 | 18 | 0 | 37 | 85 | 51 | 4 | 24 | 432 | 21 | 8 | 17 | 129 | 27 | 926 | | 3 | 0 | 1 | 2 |
| 8:45 AM | 0 | 36 | 28 | 11 | 0 | 38 | 65 | 56 | 3 | 27 | 297 | 17 | 5 | 15 | 131 | 28 | 757 | | 7 | 3 | 1 | 3 |

Peak Rolling Hour Flow Rates

| Vehicle Type | Eastbound | | | | Westbound | | | | Northbound | | | | Southbound | | | | Total |
|--------------------|-----------|------|------|-------|-----------|------|------|-------|------------|------|-------|-------|------------|------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 5 | 0 | 0 | 0 | 5 | 0 | 12 |
| Lights | 0 | 145 | 202 | 83 | 0 | 299 | 381 | 284 | 17 | 118 | 1,353 | 116 | 22 | 71 | 687 | 132 | 3,910 |
| Mediums | 0 | 2 | 2 | 1 | 0 | 3 | 4 | 3 | 0 | 2 | 20 | 7 | 0 | 1 | 22 | 0 | 67 |
| Total | 0 | 147 | 204 | 84 | 0 | 303 | 385 | 287 | 18 | 120 | 1,378 | 123 | 22 | 72 | 714 | 132 | 3,989 |



(303) 216-2439
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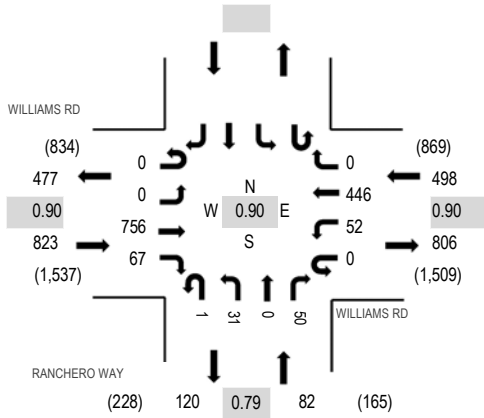
Location: 1 RANCHERO WAY & WILLIAMS RD PM

Date: Tuesday, November 6, 2018

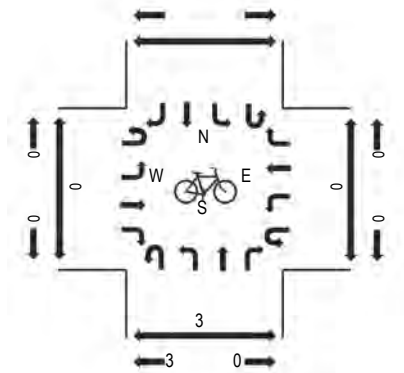
Peak Hour: 05:00 PM - 06:00 PM

Peak 15-Minutes: 05:30 PM - 05:45 PM

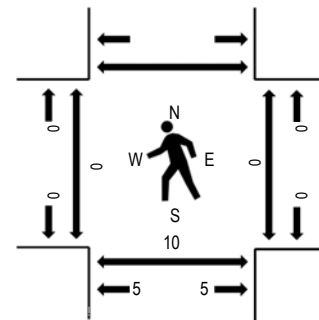
Peak Hour - All Vehicles



Peak Hour - Bicycles



Peak Hour - Pedestrians



Note: Total study counts contained in parentheses.

Traffic Counts

| Interval Start Time | WILLIAMS RD Eastbound | | | | WILLIAMS RD Westbound | | | | RANCHERO WAY Northbound | | | | Southbound | | | | Total | Rolling Hour | Pedestrian Crossings | | | |
|---------------------|-----------------------|------|------|-------|-----------------------|------|------|-------|-------------------------|------|------|-------|------------|------|------|-------|-------|--------------|----------------------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | | | West | East | South | North |
| 4:00 PM | 0 | 0 | 160 | 15 | 0 | 12 | 72 | 0 | 0 | 3 | 0 | 10 | | | | | 272 | 1,168 | 0 | 0 | 1 | |
| 4:15 PM | 0 | 0 | 157 | 17 | 0 | 16 | 78 | 0 | 0 | 8 | 0 | 18 | | | | | 294 | 1,219 | 0 | 0 | 0 | |
| 4:30 PM | 0 | 0 | 157 | 18 | 0 | 5 | 82 | 0 | 0 | 3 | 0 | 14 | | | | | 279 | 1,278 | 0 | 0 | 7 | |
| 4:45 PM | 0 | 0 | 170 | 20 | 0 | 5 | 101 | 0 | 0 | 10 | 0 | 17 | | | | | 323 | 1,387 | 0 | 0 | 1 | |
| 5:00 PM | 0 | 0 | 173 | 19 | 0 | 12 | 105 | 0 | 0 | 4 | 0 | 10 | | | | | 323 | 1,403 | 0 | 0 | 5 | |
| 5:15 PM | 0 | 0 | 187 | 16 | 0 | 16 | 111 | 0 | 0 | 7 | 0 | 16 | | | | | 353 | | 0 | 0 | 3 | |
| 5:30 PM | 0 | 0 | 216 | 12 | 0 | 14 | 125 | 0 | 1 | 8 | 0 | 12 | | | | | 388 | | 0 | 0 | 0 | |
| 5:45 PM | 0 | 0 | 180 | 20 | 0 | 10 | 105 | 0 | 0 | 12 | 0 | 12 | | | | | 339 | | 0 | 0 | 2 | |

Peak Rolling Hour Flow Rates

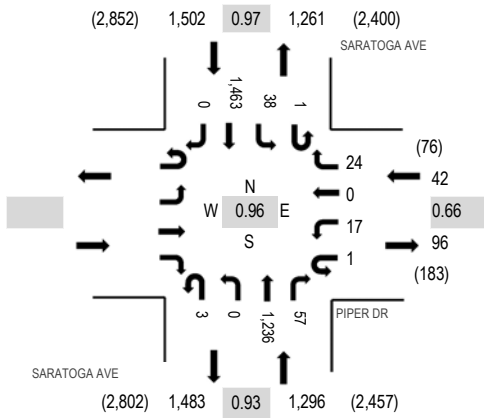
| Vehicle Type | Eastbound | | | | Westbound | | | | Northbound | | | | Southbound | | | | Total |
|--------------------|-----------|------|------|-------|-----------|------|------|-------|------------|------|------|-------|------------|------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | | | | | 0 |
| Lights | 0 | 0 | 751 | 67 | 0 | 52 | 444 | 0 | 1 | 31 | 0 | 50 | | | | | 1,396 |
| Mediums | 0 | 0 | 5 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | | | | | 7 |
| Total | 0 | 0 | 756 | 67 | 0 | 52 | 446 | 0 | 1 | 31 | 0 | 50 | | | | | 1,403 |



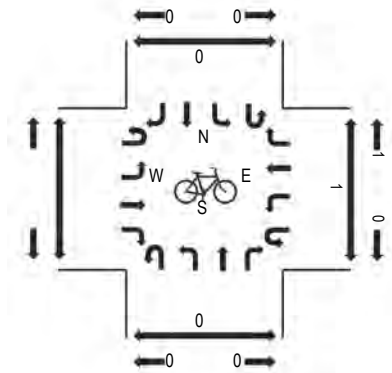
(303) 216-2439
www.alltrafficdata.net

Location: 2 SARATOGA AVE & PIPER DR PM
Date: Tuesday, November 6, 2018
Peak Hour: 05:00 PM - 06:00 PM
Peak 15-Minutes: 05:30 PM - 05:45 PM

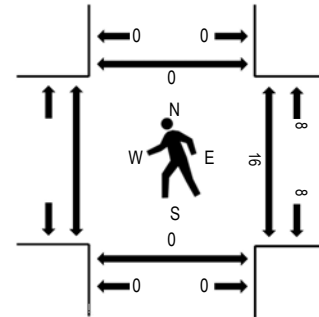
Peak Hour - All Vehicles



Peak Hour - Bicycles



Peak Hour - Pedestrians



Note: Total study counts contained in parentheses.

Traffic Counts

| Interval Start Time | Eastbound | | | | PIPER DR Westbound | | | | SARATOGA AVE Northbound | | | | SARATOGA AVE Southbound | | | | Total | Rolling Hour | Pedestrian Crossings | | | |
|---------------------|-----------|------|------|-------|--------------------|------|------|-------|-------------------------|------|------|-------|-------------------------|------|------|-------|-------|--------------|----------------------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | | | West | East | South | North |
| 4:00 PM | | | | | 0 | 5 | 0 | 2 | 0 | 0 | 277 | 9 | 2 | 13 | 276 | 0 | 584 | 2,545 | 2 | 0 | 0 | |
| 4:15 PM | | | | | 0 | 6 | 0 | 8 | 0 | 0 | 278 | 11 | 0 | 10 | 368 | 0 | 681 | 2,690 | 5 | 0 | 0 | |
| 4:30 PM | | | | | 0 | 3 | 0 | 3 | 0 | 0 | 286 | 11 | 0 | 9 | 320 | 0 | 632 | 2,697 | 2 | 0 | 0 | |
| 4:45 PM | | | | | 0 | 2 | 0 | 5 | 0 | 0 | 278 | 11 | 0 | 13 | 339 | 0 | 648 | 2,801 | 2 | 0 | 0 | |
| 5:00 PM | | | | | 0 | 3 | 0 | 8 | 1 | 0 | 330 | 17 | 1 | 7 | 362 | 0 | 729 | 2,840 | 7 | 0 | 0 | |
| 5:15 PM | | | | | 0 | 4 | 0 | 3 | 0 | 0 | 307 | 15 | 0 | 11 | 348 | 0 | 688 | | 2 | 0 | 0 | |
| 5:30 PM | | | | | 1 | 7 | 0 | 8 | 2 | 0 | 313 | 17 | 0 | 8 | 380 | 0 | 736 | | 4 | 0 | 0 | |
| 5:45 PM | | | | | 0 | 3 | 0 | 5 | 0 | 0 | 286 | 8 | 0 | 12 | 373 | 0 | 687 | | 3 | 0 | 0 | |

Peak Rolling Hour Flow Rates

| Vehicle Type | Eastbound | | | | Westbound | | | | Northbound | | | | Southbound | | | | Total |
|--------------------|-----------|------|------|-------|-----------|------|------|-------|------------|------|-------|-------|------------|------|-------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | |
| Articulated Trucks | | | | | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 4 |
| Lights | | | | | 1 | 17 | 0 | 23 | 3 | 0 | 1,220 | 56 | 1 | 38 | 1,451 | 0 | 2,810 |
| Mediums | | | | | 0 | 0 | 0 | 1 | 0 | 0 | 12 | 1 | 0 | 0 | 12 | 0 | 26 |
| Total | | | | | 1 | 17 | 0 | 24 | 3 | 0 | 1,236 | 57 | 1 | 38 | 1,463 | 0 | 2,840 |



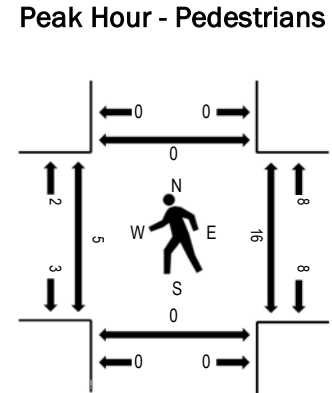
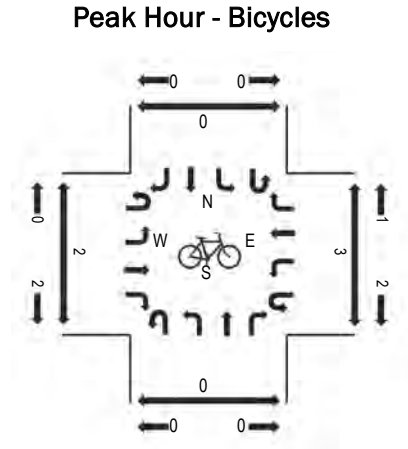
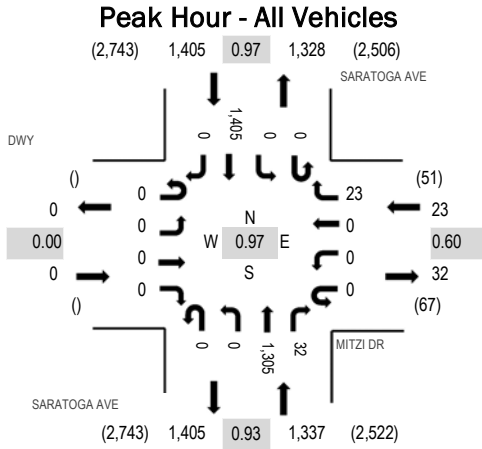
(303) 216-2439
www.alltrafficdata.net

Location: 3 SARATOGA AVE & MITZI DR PM

Date: Tuesday, November 6, 2018

Peak Hour: 04:45 PM - 05:45 PM

Peak 15-Minutes: 05:00 PM - 05:15 PM



Note: Total study counts contained in parentheses.

Traffic Counts

| Interval Start Time | DWY Eastbound | | | | MITZI DR Westbound | | | | SARATOGA AVE Northbound | | | | SARATOGA AVE Southbound | | | | Total | Rolling Hour | Pedestrian Crossings | | | | |
|---------------------|---------------|------|------|-------|--------------------|------|------|-------|-------------------------|------|------|-------|-------------------------|------|------|-------|-------|--------------|----------------------|------|-------|-------|---|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | | | West | East | South | North | |
| 4:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 0 | 0 | 266 | 10 | 0 | 0 | 327 | 0 | 612 | 2,567 | 1 | 3 | 0 | 0 |
| 4:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 0 | 288 | 9 | 0 | 0 | 340 | 0 | 649 | 2,667 | 1 | 4 | 0 | 0 |
| 4:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 299 | 6 | 0 | 0 | 329 | 0 | 638 | 2,730 | 0 | 2 | 0 | 0 |
| 4:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 306 | 5 | 0 | 0 | 354 | 0 | 668 | 2,765 | 2 | 1 | 0 | 0 |
| 5:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 336 | 4 | 0 | 0 | 362 | 0 | 712 | 2,749 | 2 | 6 | 0 | 0 |
| 5:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 347 | 12 | 0 | 0 | 347 | 0 | 712 | | 0 | 4 | 0 | 0 |
| 5:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 316 | 11 | 0 | 0 | 342 | 0 | 673 | | 1 | 5 | 0 | 0 |
| 5:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 297 | 10 | 0 | 0 | 342 | 0 | 652 | | 1 | 4 | 2 | 0 |

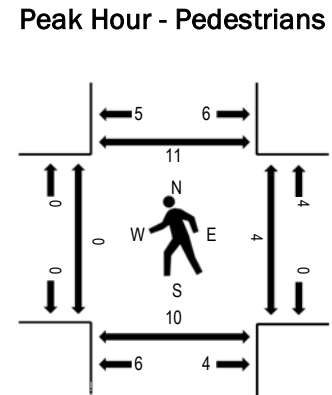
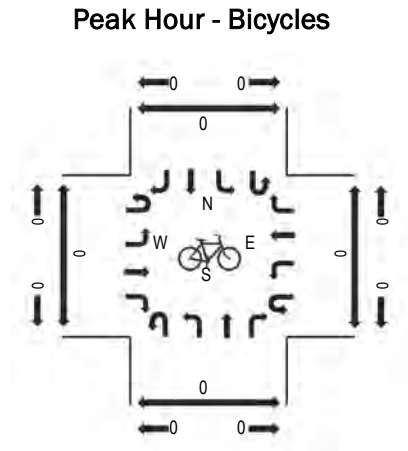
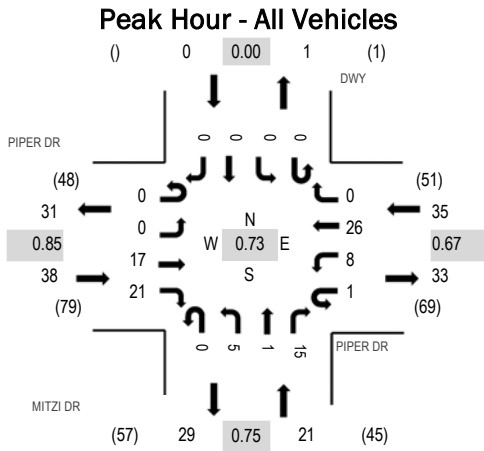
Peak Rolling Hour Flow Rates

| Vehicle Type | Eastbound | | | | Westbound | | | | Northbound | | | | Southbound | | | | Total |
|--------------------|-----------|------|------|-------|-----------|------|------|-------|------------|------|-------|-------|------------|------|-------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 1 | 0 | 7 |
| Lights | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 1,286 | 32 | 0 | 0 | 1,397 | 0 | 2,738 |
| Mediums | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 13 | 0 | 0 | 0 | 7 | 0 | 20 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 1,305 | 32 | 0 | 0 | 1,405 | 0 | 2,765 |



(303) 216-2439
www.alltrafficdata.net

Location: 4 MITZI DR & PIPER DR PM
Date: Tuesday, November 6, 2018
Peak Hour: 05:00 PM - 06:00 PM
Peak 15-Minutes: 05:45 PM - 06:00 PM



Note: Total study counts contained in parentheses.

Traffic Counts

| Interval Start Time | PIPER DR Eastbound | | | | PIPER DR Westbound | | | | MITZI DR Northbound | | | | DWY Southbound | | | | Total | Rolling Hour | Pedestrian Crossings | | | |
|---------------------|--------------------|------|------|-------|--------------------|------|------|-------|---------------------|------|------|-------|----------------|------|------|-------|-------|--------------|----------------------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | | | West | East | South | North |
| 4:00 PM | 0 | 0 | 9 | 3 | 0 | 0 | 2 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 18 | 81 | 0 | 0 | 3 | 0 |
| 4:15 PM | 1 | 0 | 5 | 5 | 0 | 2 | 1 | 0 | 0 | 2 | 0 | 5 | 0 | 0 | 0 | 0 | 21 | 81 | 0 | 0 | 1 | 5 |
| 4:30 PM | 0 | 0 | 4 | 5 | 0 | 4 | 3 | 0 | 0 | 2 | 0 | 3 | 0 | 0 | 0 | 0 | 21 | 84 | 0 | 0 | 1 | 0 |
| 4:45 PM | 0 | 0 | 3 | 6 | 0 | 3 | 1 | 0 | 0 | 3 | 0 | 5 | 0 | 0 | 0 | 0 | 21 | 83 | 0 | 0 | 1 | 0 |
| 5:00 PM | 0 | 0 | 1 | 8 | 0 | 1 | 7 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 18 | 94 | 0 | 3 | 1 | 3 |
| 5:15 PM | 0 | 0 | 8 | 3 | 0 | 5 | 2 | 0 | 0 | 2 | 0 | 4 | 0 | 0 | 0 | 0 | 24 | | 0 | 0 | 3 | 0 |
| 5:30 PM | 0 | 0 | 3 | 5 | 1 | 0 | 6 | 0 | 0 | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 20 | | 0 | 0 | 5 | 0 |
| 5:45 PM | 0 | 0 | 5 | 5 | 0 | 2 | 11 | 0 | 0 | 2 | 1 | 6 | 0 | 0 | 0 | 0 | 32 | | 0 | 1 | 1 | 8 |

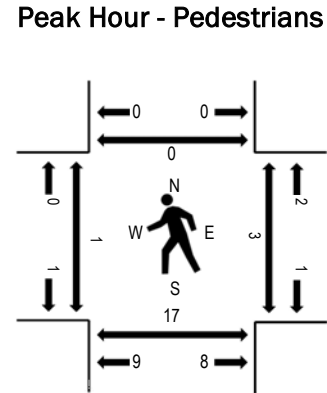
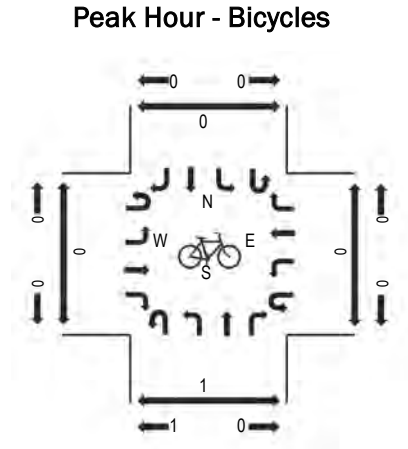
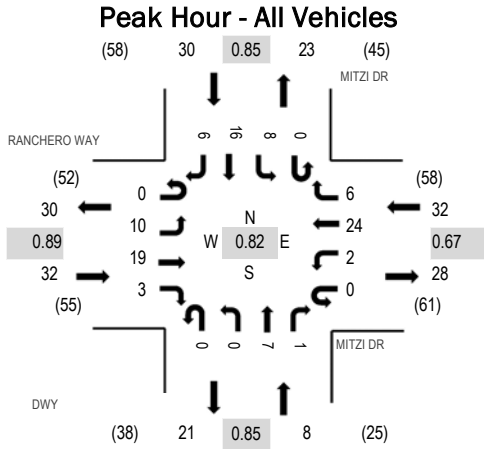
Peak Rolling Hour Flow Rates

| Vehicle Type | Eastbound | | | | Westbound | | | | Northbound | | | | Southbound | | | | Total |
|--------------------|-----------|------|------|-------|-----------|------|------|-------|------------|------|------|-------|------------|------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lights | 0 | 0 | 17 | 21 | 1 | 8 | 26 | 0 | 0 | 5 | 1 | 14 | 0 | 0 | 0 | 0 | 93 |
| Mediums | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 1 |
| Total | 0 | 0 | 17 | 21 | 1 | 8 | 26 | 0 | 0 | 5 | 1 | 15 | 0 | 0 | 0 | 0 | 94 |



(303) 216-2439
www.alltrafficdata.net

Location: 5 DWY & MITZI DR PM
Date: Tuesday, November 6, 2018
Peak Hour: 05:00 PM - 06:00 PM
Peak 15-Minutes: 05:15 PM - 05:30 PM



Note: Total study counts contained in parentheses.

Traffic Counts

| Interval Start Time | RANCHERO WAY Eastbound | | | | MITZI DR Westbound | | | | DWY Northbound | | | | MITZI DR Southbound | | | | Total | Rolling Hour | Pedestrian Crossings | | | |
|---------------------|------------------------|------|------|-------|--------------------|------|------|-------|----------------|------|------|-------|---------------------|------|------|-------|-------|--------------|----------------------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | | | West | East | South | North |
| 4:00 PM | 0 | 1 | 4 | 1 | 1 | 0 | 5 | 1 | 0 | 2 | 2 | 0 | 0 | 2 | 0 | 0 | 19 | 94 | 0 | 0 | 2 | 0 |
| 4:15 PM | 0 | 2 | 3 | 0 | 0 | 4 | 3 | 4 | 1 | 1 | 0 | 2 | 0 | 2 | 3 | 2 | 27 | 97 | 0 | 2 | 5 | 0 |
| 4:30 PM | 0 | 0 | 6 | 1 | 0 | 0 | 2 | 1 | 0 | 1 | 3 | 0 | 0 | 5 | 3 | 2 | 24 | 101 | 0 | 1 | 2 | 0 |
| 4:45 PM | 0 | 2 | 3 | 0 | 0 | 0 | 2 | 3 | 0 | 0 | 3 | 2 | 0 | 3 | 4 | 2 | 24 | 101 | 2 | 0 | 2 | 0 |
| 5:00 PM | 0 | 1 | 4 | 3 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 4 | 2 | 22 | 102 | 0 | 0 | 3 | 0 |
| 5:15 PM | 0 | 2 | 7 | 0 | 0 | 1 | 8 | 3 | 0 | 0 | 2 | 1 | 0 | 2 | 5 | 0 | 31 | | 1 | 0 | 6 | 0 |
| 5:30 PM | 0 | 3 | 6 | 0 | 0 | 0 | 5 | 1 | 0 | 0 | 3 | 0 | 0 | 1 | 3 | 2 | 24 | | 0 | 1 | 7 | 0 |
| 5:45 PM | 0 | 4 | 2 | 0 | 0 | 1 | 5 | 2 | 0 | 0 | 2 | 0 | 0 | 3 | 4 | 2 | 25 | | 0 | 2 | 1 | 0 |

Peak Rolling Hour Flow Rates

| Vehicle Type | Eastbound | | | | Westbound | | | | Northbound | | | | Southbound | | | | Total |
|--------------------|-----------|------|------|-------|-----------|------|------|-------|------------|------|------|-------|------------|------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lights | 0 | 10 | 19 | 3 | 0 | 2 | 24 | 6 | 0 | 0 | 6 | 1 | 0 | 8 | 16 | 6 | 101 |
| Mediums | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 |
| Total | 0 | 10 | 19 | 3 | 0 | 2 | 24 | 6 | 0 | 0 | 7 | 1 | 0 | 8 | 16 | 6 | 102 |



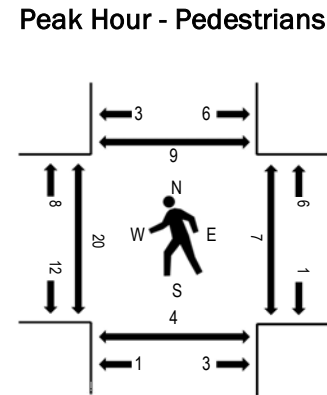
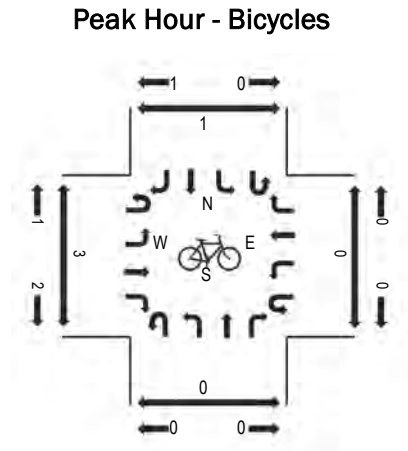
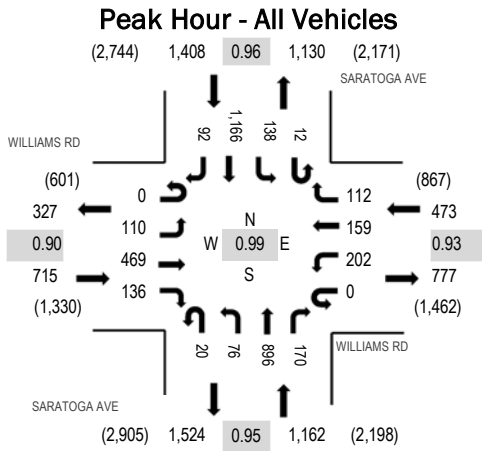
(303) 216-2439
www.alltrafficdata.net

Location: 6 SARATOGA AVE & WILLIAMS RD PM

Date: Tuesday, November 6, 2018

Peak Hour: 04:45 PM - 05:45 PM

Peak 15-Minutes: 05:00 PM - 05:15 PM



Note: Total study counts contained in parentheses.

Traffic Counts

| Interval Start Time | WILLIAMS RD Eastbound | | | | WILLIAMS RD Westbound | | | | SARATOGA AVE Northbound | | | | SARATOGA AVE Southbound | | | | Total | Rolling Hour | Pedestrian Crossings | | | |
|---------------------|-----------------------|------|------|-------|-----------------------|------|------|-------|-------------------------|------|------|-------|-------------------------|------|------|-------|-------|--------------|----------------------|------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | | | West | East | South | North |
| 4:00 PM | 0 | 23 | 115 | 24 | 0 | 28 | 23 | 52 | 2 | 15 | 161 | 29 | 8 | 32 | 261 | 20 | 793 | 3,412 | 5 | 3 | 8 | 2 |
| 4:15 PM | 0 | 16 | 95 | 19 | 0 | 38 | 24 | 21 | 4 | 26 | 230 | 29 | 6 | 40 | 312 | 17 | 877 | 3,571 | 2 | 1 | 1 | 3 |
| 4:30 PM | 0 | 26 | 99 | 23 | 0 | 40 | 30 | 20 | 3 | 14 | 203 | 28 | 3 | 30 | 278 | 26 | 823 | 3,635 | 2 | 5 | 11 | 4 |
| 4:45 PM | 0 | 29 | 112 | 28 | 0 | 44 | 37 | 28 | 5 | 18 | 202 | 41 | 1 | 30 | 321 | 23 | 919 | 3,758 | 3 | 3 | 0 | 0 |
| 5:00 PM | 0 | 31 | 101 | 44 | 0 | 50 | 30 | 25 | 8 | 21 | 250 | 33 | 3 | 39 | 289 | 28 | 952 | 3,727 | 2 | 3 | 3 | 2 |
| 5:15 PM | 0 | 27 | 134 | 39 | 0 | 53 | 44 | 32 | 5 | 18 | 217 | 52 | 6 | 35 | 255 | 24 | 941 | | 8 | 1 | 1 | 5 |
| 5:30 PM | 0 | 23 | 122 | 25 | 0 | 55 | 48 | 27 | 2 | 19 | 227 | 44 | 2 | 34 | 301 | 17 | 946 | | 7 | 0 | 0 | 2 |
| 5:45 PM | 0 | 14 | 125 | 36 | 0 | 48 | 44 | 26 | 7 | 17 | 230 | 38 | 2 | 25 | 258 | 18 | 888 | | 7 | 0 | 0 | 3 |

Peak Rolling Hour Flow Rates

| Vehicle Type | Eastbound | | | | Westbound | | | | Northbound | | | | Southbound | | | | Total |
|--------------------|-----------|------|------|-------|-----------|------|------|-------|------------|------|------|-------|------------|------|-------|-------|-------|
| | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 2 |
| Lights | 0 | 110 | 464 | 136 | 0 | 201 | 157 | 112 | 20 | 76 | 883 | 168 | 12 | 137 | 1,157 | 92 | 3,725 |
| Mediums | 0 | 0 | 5 | 0 | 0 | 1 | 2 | 0 | 0 | 0 | 12 | 2 | 0 | 1 | 8 | 0 | 31 |
| Total | 0 | 110 | 469 | 136 | 0 | 202 | 159 | 112 | 20 | 76 | 896 | 170 | 12 | 138 | 1,166 | 92 | 3,758 |

Appendix C

Level of Service Calculations

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Existing AM

Intersection #1: Ranchoero Way/Williams Road

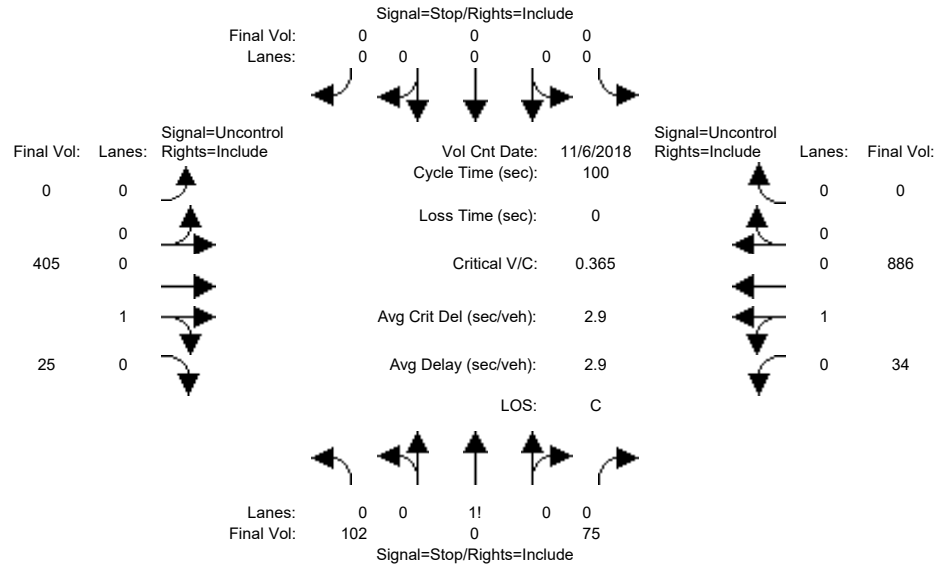


Table with columns: Street Name, Approach, Movement, Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Table with columns: Critical Gap Module, Critical Gap, FollowUpTim.

Table with columns: Capacity Module, Cnflct Vol, Potent Cap., Move Cap., Total Cap, Volume/Cap.

Table with columns: Level Of Service Module, ZWay95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Table with columns: Note: Queue reported is the number of cars per lane, HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, Time Period, Upstream Signals, Link Index, Dist(miles), Speed(mph), SignalIndex, Cycle Time, InitVolume, Saturation, ArrivalType, G/C, Computation 1, Computation 2, Computation 3, Computation 4, Computation 5.

```

InitPotCap: 163 147 640 117 145 346 1636 xxxxxx xxxxxx 1140 xxxxxx xxxxxx
UpstreamAdj:1.00 1.000 1.000 1.00 1.000 1.000 1.000 1.00 x.xxx x.xxx 1.00 x.xxx x.xxx
Potent Cap.: 163 147 640 117 145 346 1636 xxxxxx xxxxxx 1140 xxxxxx xxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Two-Stage Gap Acceptance [Median Type: TWLTL][Median Storage: 1 car]
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Two-Stage Gap Acceptance - Stage One Module:
InitCfltlVol: 418 418 xxxxxx 954 954 xxxxxx xxxxx xxxxxx xxxxxx xxxxxx xxxxxx
UpstreamSat: 0 0 xxxxxx 0 0 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Cnflct Vol: 418 418 xxxxxx 954 954 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
InitPotCap: 669 594 xxxxxx 313 340 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Potent Cap.: 669 594 xxxxxx 313 340 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Move Cap.: 669 594 xxxxxx 304 330 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Two-Stage Gap Acceptance - Stage Two Module:
InitCfltlVol: 954 954 xxxxxx 455 430 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamSat: 0 0 xxxxxx 0 0 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Cnflct Vol: 954 954 xxxxxx 455 430 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
InitPotCap: 377 340 xxxxxx 589 587 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Potent Cap.: 377 340 xxxxxx 589 587 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Move Cap.: 366 330 xxxxxx 520 587 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Peak Hour Delay Signal Warrant Report

```

```

*****
Intersection #1 Ranchero Way/Williams Road
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 0 0 1 0 0 0
Initial Vol: 102 0 75 0 0 0 0 0 405 25 34 886 0
ApproachDel: 23.6 xxxxxxx xxxxxxx xxxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=1.2]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=177]
SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=1527]
SUCCEED - Total volume greater than or equal to 650 for intersection
with less than four approaches.
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

```

*****
Intersection #1 Ranchero Way/Williams Road
*****
Future Volume Alternative: Peak Hour Warrant Met
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 0 1 0 0 0
Initial Vol: 102 0 75 0 0 0 0 0 405 25 34 886 0
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Major Street Volume: 1350
Minor Approach Volume: 177
Minor Approach Volume Threshold: 139
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Existing AM

Intersection #2: Saratoga Avenue/Piper Drive

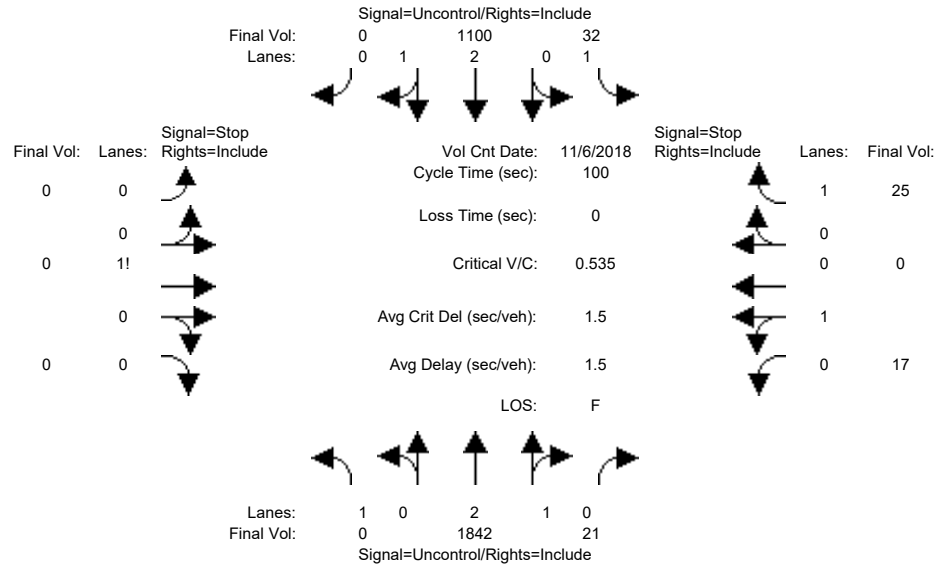


Table with columns for Street Name, Approach, Movement, Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters.

Table for Capacity Module showing Conflict Vol, Potent Cap, Move Cap, and Volume/Cap.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Table with various performance metrics including HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, Time Period, Upstream Signals, Link Index, Dist(miles), Speed (mph), SignalIndex, Cycle Time, InitVolume, Saturation, ArrivalType, G/C, and various computation results.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
 Potent Cap.:1636 xxxxx xxxxx 328 xxxxx xxxxx 53 13 636 34 13 433
 Peak Hour Delay Signal Warrant Report

 Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met
 -----|-----|-----|-----|-----|
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 -----|-----|-----|-----|-----|
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 1 0 2 1 0 1 0 2 1 0 0 0 1 0 0 0 1 0 0 1
 Initial Vol: 0 1842 21 32 1100 0 0 0 0 0 17 0 25
 ApproachDel: xxxxxx xxxxxx xxxxxx 93.0
 -----|-----|-----|-----|-----|

Approach[westbound][lanes=2][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=1.1]
 FAIL - Vehicle-hours less than 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=42]
 FAIL - Approach volume less than 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=3037]
 SUCCEEDED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met
 -----|-----|-----|-----|-----|
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 -----|-----|-----|-----|-----|
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 1 0 2 1 0 1 0 2 1 0 0 0 1 0 0 0 1 0 0 1
 Initial Vol: 0 1842 21 32 1100 0 0 0 0 0 17 0 25
 -----|-----|-----|-----|-----|

Major Street Volume: 2995
 Minor Approach Volume: 42
 Minor Approach Volume Threshold: -98 [less than minimum of 150]

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Existing AM

Intersection #3: Saratoga Avenue/Mitzi Drive

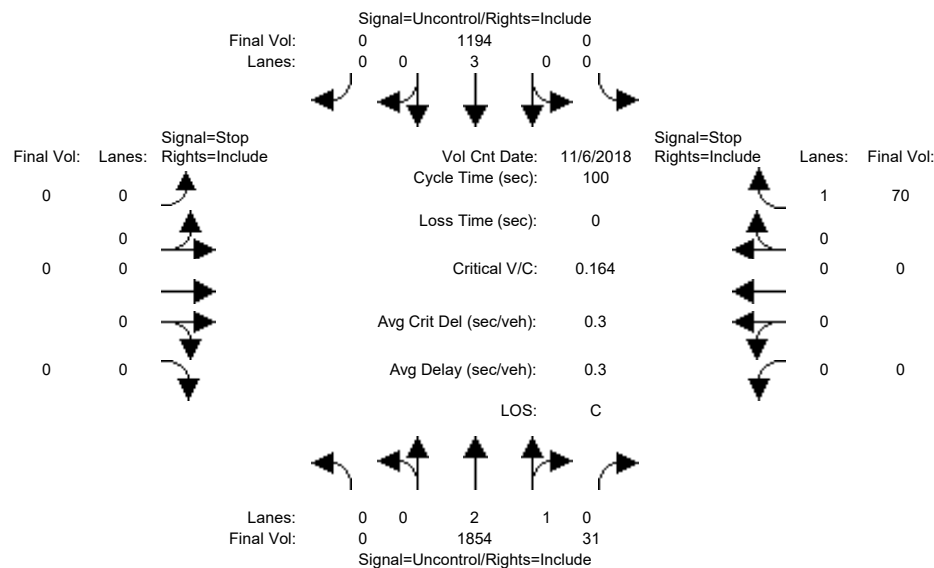


Table with columns: Street Name, Approach, Movement, Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume. Rows for Saratoga Avenue and Mitzi Drive.

Table for Critical Gap Module with columns: Critical Gap, FollowUpTim, and values.

Table for Capacity Module with columns: Cnflct Vol, Potent Cap, Move Cap, Volume/Cap and values.

Table for Level Of Service Module with columns: ZWay95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS and values.

Table with columns: HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, Time Period and values.

Upstream Signals: Link Index: #42, Dist(miles): 0.000, Speed (mph): 0.00, SignalIndex: #3793, Cycle Time: 0 secs, InitVolume: 0 0, Saturation: 0 0, ArrivalType: 0 0, G/C: 0.00 0.00, Computation 1: Time for Queue to Clear at Each Upstream Intersection, Computation 2: Time Intersection Blocked Because of Upstream Platoons, Computation 3: Platoon Event Periods, Computation 4: Conflicting Flows During Each Unblocked Period, Computation 5: Capacity for Subject Movement During Unblocked Period.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
 Potent Cap.:1636 xxxxxx xxxxxx 1636 xxxxxx xxxxxx 50 12 607 35 13 427
 Peak Hour Delay Signal Warrant Report

 Intersection #3 Saratoga Avenue/Mitzi Drive

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 0 0 2 1 0 0 0 3 0 0 0 0 0 0 0 0 0 0 0 1
 Initial Vol: 0 1854 31 0 1194 0 0 0 0 0 0 0 0 0 0 70
 ApproachDel: xxxxxx xxxxxx xxxxxx 15.1

 Approach[westbound][lanes=1][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.3]
 FAIL - Vehicle-hours less than 4 for one lane approach.
 Signal Warrant Rule #2: [approach volume=70]
 FAIL - Approach volume less than 100 for one lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=3149]
 SUCCEEDED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #3 Saratoga Avenue/Mitzi Drive

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 0 0 2 1 0 0 0 3 0 0 0 0 0 0 0 0 0 0 0 1
 Initial Vol: 0 1854 31 0 1194 0 0 0 0 0 0 0 0 0 0 70

 Major Street Volume: 3079
 Minor Approach Volume: 70
 Minor Approach Volume Threshold: -103 [less than minimum of 100]

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Existing AM

Intersection #4: Mitzi Drive/Piper Drive

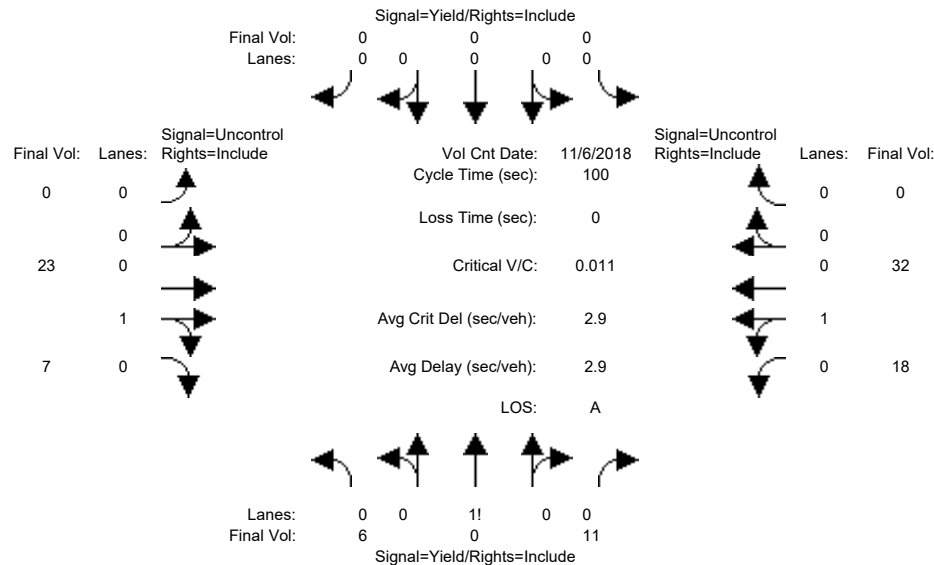


Table with columns for Street Name, Approach, Movement, Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters.

Table for Capacity Module showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Table for Note: Queue reported is the number of cars per lane, including HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, and Time Period.

Peak Hour Delay Signal Warrant Report
Intersection #4 Mitzi Drive/Piper Drive
Future Volume Alternative: Peak Hour Warrant NOT Met

Table for Future Volume Alternative: Peak Hour Warrant NOT Met, showing Approach, Movement, Control, Lanes, Initial Vol, and ApproachDel.

Approach[northbound][lanes=1][control=Yield Sign]
Signal Warrant Rule #1: [vehicle-hours=0.0]
FAIL - Controller not stop sign.
Signal Warrant Rule #2: [approach volume=17]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=97]
FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

```

*****
Intersection #4 Mitzi Drive/Piper Drive
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|-----|
Control:   Yield Sign      Yield Sign      Uncontrolled    Uncontrolled
Lanes:     0 0 1 0 0      0 0 0 0 0      0 0 0 1 0      0 1 0 0 0
Initial Vol: 6  0  11      0  0  0  0      0  23  7      18  32  0
-----|-----|-----|-----|-----|
Major Street Volume:      80
Minor Approach Volume:    17
Minor Approach Volume Threshold: 893
-----|-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Existing AM

Intersection #5: Mitzi Drive/Ranchero Way

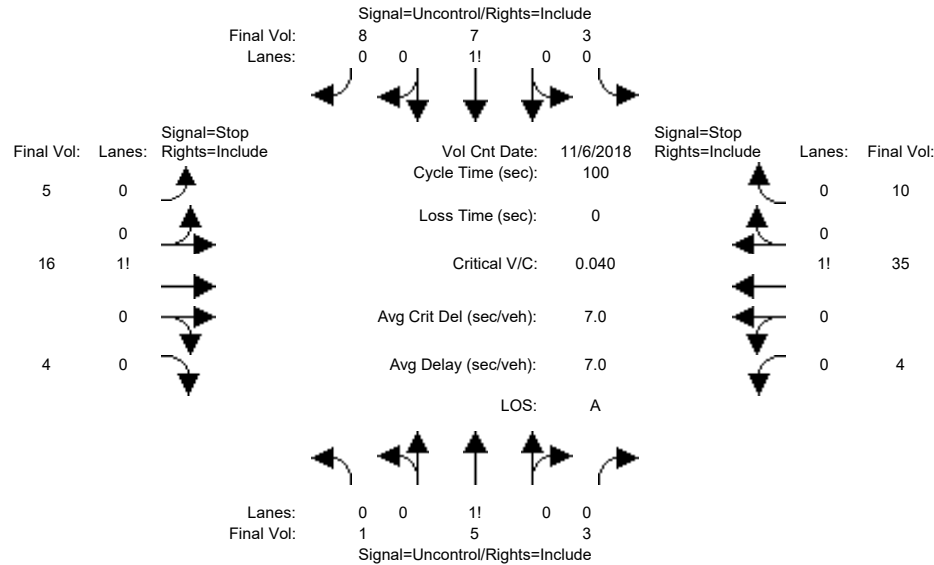


Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters for different approaches.

Table for Capacity Module showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. for various movements.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Table with traffic metrics including HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, and Time Period.

Peak Hour Delay Signal Warrant Report
Intersection #5 Mitzi Drive/Ranchero Way
Future Volume Alternative: Peak Hour Warrant NOT Met

Table for Signal Warrant analysis showing Approach, Control, Lanes, Initial Vol, and ApproachDel for Eastbound and Westbound directions.

Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=25]
FAIL - Approach volume less than 100 for one lane approach.

Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=49]
FAIL - Approach volume less than 100 for one lane approach.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting

a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

```

*****
Intersection #5 Mitzi Drive/Ranchero Way
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|-----|
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R        L - T - R        L - T - R        L - T - R
-----|-----|-----|-----|-----|-----|
Control:   Uncontrolled    Uncontrolled    Stop Sign       Stop Sign
Lanes:     0 0 1! 0 0          0 0 1! 0 0          0 0 1! 0 0          0 0 1! 0 0
Initial Vol: 1 5 3          3 7 8            5 16 4            4 4 35 10
-----|-----|-----|-----|-----|-----|
Major Street Volume:          27
Minor Approach Volume:        49
Minor Approach Volume Threshold: 1183
-----|-----|-----|-----|-----|

```

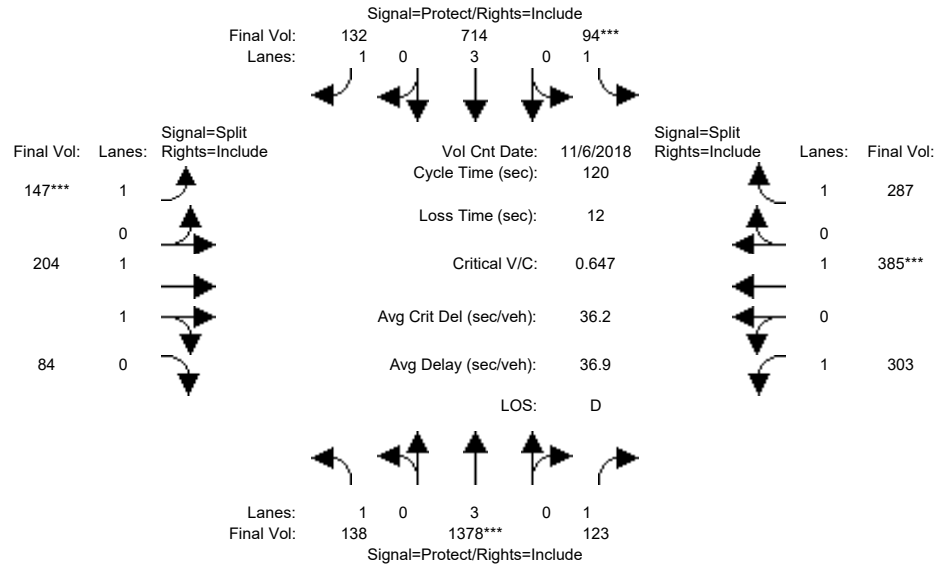
SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
 2000 HCM Operations (Future Volume Alternative)
 Existing AM

Intersection #3793: Saratoga Avenue/Williams Road



| Street Name: | Saratoga Avenue | | | | | | Williams Road | | | | | |
|---|-----------------|------|------|-------------|------|------|---------------|------|------|------------|------|------|
| | North Bound | | | South Bound | | | East Bound | | | West Bound | | |
| Movement: | L | T | R | L | T | R | L | T | R | L | T | R |
| Min. Green: | 7 | 10 | 10 | 7 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 |
| Y+R: | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Volume Module: >> Count Date: 6 Nov 2018 << 7:30 AM - 8:30 AM | 138 | 1378 | 123 | 94 | 714 | 132 | 147 | 204 | 84 | 303 | 385 | 287 |
| Base Vol: | 138 | 1378 | 123 | 94 | 714 | 132 | 147 | 204 | 84 | 303 | 385 | 287 |
| Growth Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Initial Bse: | 138 | 1378 | 123 | 94 | 714 | 132 | 147 | 204 | 84 | 303 | 385 | 287 |
| Added Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| PasserByVol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Fut: | 138 | 1378 | 123 | 94 | 714 | 132 | 147 | 204 | 84 | 303 | 385 | 287 |
| User Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Volume: | 138 | 1378 | 123 | 94 | 714 | 132 | 147 | 204 | 84 | 303 | 385 | 287 |
| Reduct Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced Vol: | 138 | 1378 | 123 | 94 | 714 | 132 | 147 | 204 | 84 | 303 | 385 | 287 |
| PCE Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| MLF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Final Volume: | 138 | 1378 | 123 | 94 | 714 | 132 | 147 | 204 | 84 | 303 | 385 | 287 |
| Saturation Flow Module: | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Sat/Lane: | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Adjustment: | 0.92 | 1.00 | 0.92 | 0.92 | 1.00 | 0.92 | 0.92 | 0.98 | 0.95 | 0.92 | 1.00 | 0.92 |
| Lanes: | 1.00 | 3.00 | 1.00 | 1.00 | 3.00 | 1.00 | 1.00 | 1.40 | 0.60 | 1.00 | 1.00 | 1.00 |
| Final Sat.: | 1750 | 5700 | 1750 | 1750 | 5700 | 1750 | 1750 | 2620 | 1079 | 1750 | 1900 | 1750 |
| Capacity Analysis Module: | 0.08 | 0.24 | 0.07 | 0.05 | 0.13 | 0.08 | 0.08 | 0.08 | 0.08 | 0.17 | 0.20 | 0.16 |
| Vol/Sat: | 0.08 | 0.24 | 0.07 | 0.05 | 0.13 | 0.08 | 0.08 | 0.08 | 0.08 | 0.17 | 0.20 | 0.16 |
| Crit Moves: | *** | *** | *** | *** | *** | *** | *** | *** | *** | *** | *** | *** |
| Green Time: | 21.2 | 44.9 | 44.9 | 10.0 | 33.6 | 33.6 | 15.6 | 15.6 | 15.6 | 37.6 | 37.6 | 37.6 |
| Volume/Cap: | 0.45 | 0.65 | 0.19 | 0.65 | 0.45 | 0.27 | 0.65 | 0.60 | 0.60 | 0.55 | 0.65 | 0.52 |
| Uniform Del: | 44.2 | 31.0 | 25.3 | 53.3 | 35.5 | 33.6 | 49.6 | 49.3 | 49.3 | 34.2 | 35.5 | 33.8 |
| IncrementDel: | 1.0 | 0.7 | 0.1 | 9.7 | 0.2 | 0.3 | 6.3 | 2.1 | 2.1 | 1.2 | 2.5 | 0.9 |
| InitQueueDel: | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Delay Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Delay/Veh: | 45.2 | 31.7 | 25.4 | 63.0 | 35.7 | 33.9 | 55.9 | 51.4 | 51.4 | 35.4 | 38.0 | 34.8 |
| User DelAdj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| AdjDel/Veh: | 45.2 | 31.7 | 25.4 | 63.0 | 35.7 | 33.9 | 55.9 | 51.4 | 51.4 | 35.4 | 38.0 | 34.8 |
| LOS by Move: | D | C | C | E | D | C | E | D | D | D | D | C |
| HCM2k95thQ: | 10 | 25 | 6 | 9 | 14 | 8 | 13 | 12 | 12 | 19 | 23 | 18 |

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Existing PM

Intersection #1: Ranchoero Way/Williams Road

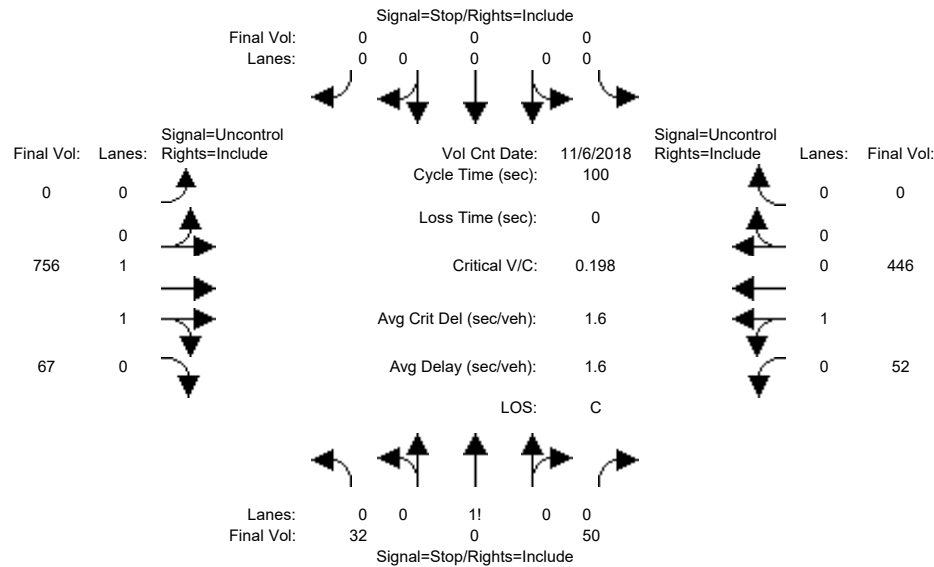


Table with columns for Street Name, Approach, Movement, Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other metrics.

Table for Capacity Module showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Table with columns for HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, Time Period.

Upstream Signals: Link Index, Dist(miles), Speed(mph), SignalIndex, Cycle Time, InitVolume, Saturation, ArrivalType, G/C, Computation 1, Computation 2, Computation 3, Computation 4, Computation 5.

UpstreamAdj:1.00 1.000 1.000 1.00 1.000 1.000 1.00 x.xxx x.xxx 1.00 x.xxx x.xxx
 Potent Cap.: 170 154 645 250 147 617 1636 xxxxx xxxxx 816 xxxxx xxxxx
 Peak Hour Delay Signal Warrant Report

 Intersection #1 Rancho Way/Williams Road

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
 Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 0 1 1 0 0 1 0 0 0
 Initial Vol: 32 0 50 0 0 0 0 0 756 67 52 446 0
 ApproachDel: 21.6 xxxxxx xxxxxx xxxxxx

 Approach[northbound][lanes=1][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.5]
 FAIL - Vehicle-hours less than 4 for one lane approach.
 Signal Warrant Rule #2: [approach volume=82]
 FAIL - Approach volume less than 100 for one lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=1403]
 SUCCEED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

 SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #1 Rancho Way/Williams Road

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
 Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 0 1 1 0 0 1 0 0 0
 Initial Vol: 32 0 50 0 0 0 0 0 756 67 52 446 0

 Major Street Volume: 1321
 Minor Approach Volume: 82
 Minor Approach Volume Threshold: 189

 SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Existing PM

Intersection #2: Saratoga Avenue/Piper Drive

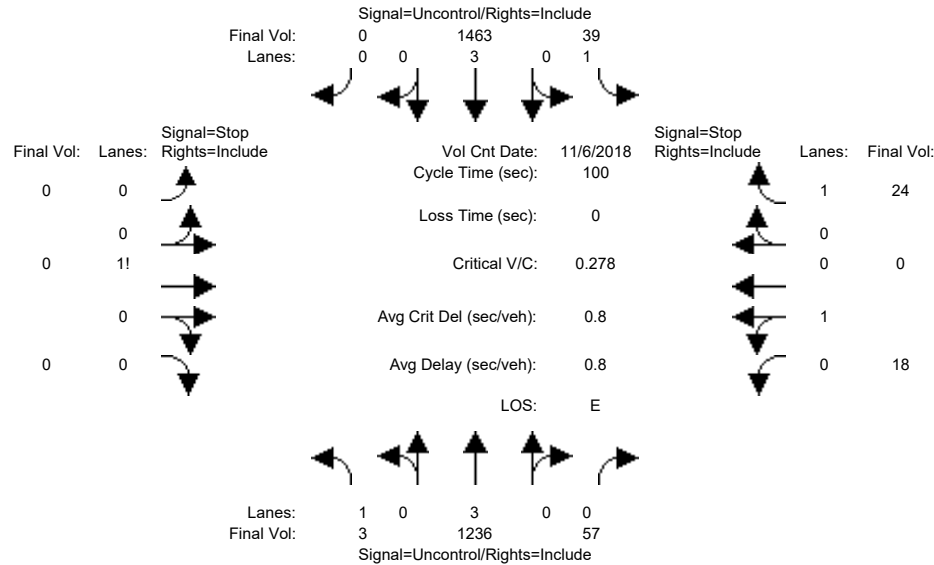


Table with columns for Street Name, Approach, Movement, Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume. It details traffic volumes for Saratoga Avenue and Piper Drive.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters for different movements.

Table for Capacity Module showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. for various movements.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Table for Note: Queue reported is the number of cars per lane. Includes HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, and Time Period.

Table for Upstream Signals and Computation 1-5. Includes Link Index, Dist(miles), Speed (mph), SignalIndex, Cycle Time, InitVolume, Saturation, ArrivalType, G/C, and various computation results for queue clearing and platoon blocking.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
 Potent Cap.: 468 xxxxx xxxxx 543 xxxxx xxxxx 39 18 531 69 18 570
 Peak Hour Delay Signal Warrant Report

 Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met
 -----|-----|-----|-----|-----|
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 -----|-----|-----|-----|-----|
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 1 0 2 1 0 1 0 3 0 0 0 0 1 0 0 0 1 0 0 1
 Initial Vol: 3 1236 57 39 1463 0 0 0 0 0 18 0 24
 ApproachDel: xxxxxx xxxxxx xxxxxx 41.3
 -----|-----|-----|-----|-----|

Approach[westbound][lanes=2][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.5]
 FAIL - Vehicle-hours less than 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=42]
 FAIL - Approach volume less than 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=2840]
 SUCCEED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met
 -----|-----|-----|-----|-----|
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 -----|-----|-----|-----|-----|
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 1 0 2 1 0 1 0 3 0 0 0 0 1 0 0 0 1 0 0 1
 Initial Vol: 3 1236 57 39 1463 0 0 0 0 0 18 0 24
 -----|-----|-----|-----|-----|

Major Street Volume: 2798
 Minor Approach Volume: 42
 Minor Approach Volume Threshold: -68 [less than minimum of 150]

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Level of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Existing PM

Intersection #3: Saratoga Avenue/Mitzi Drive

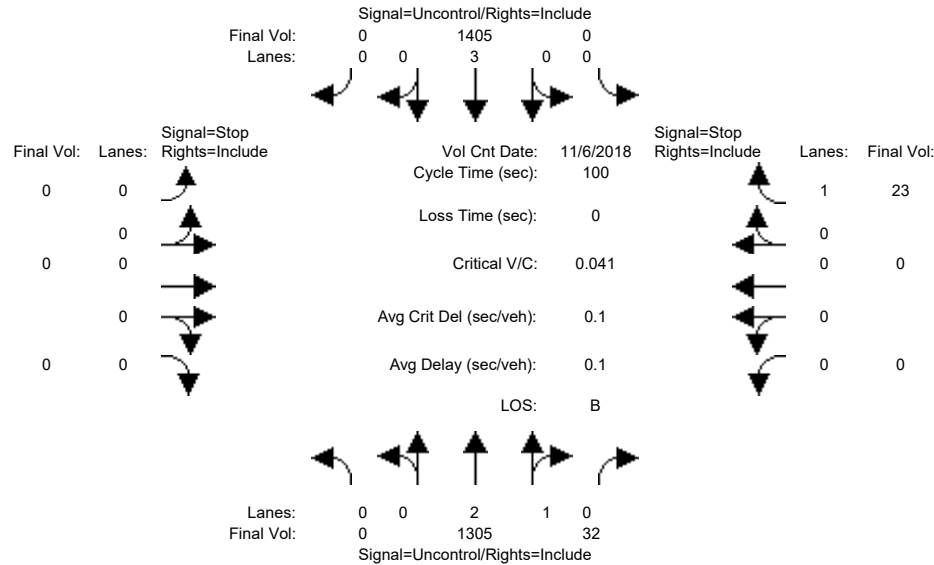


Table with columns for Street Name, Approach, Movement, Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other metrics.

Table for Capacity Module showing Cnflct Vol, Potent Cap, Move Cap, and Volume/Cap.

Table for Level of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.
HevVeh: 0%
Grade: 0%
Peds/Hour: 0
Pedestrian Walk Speed: 4.00 feet/sec
LaneWidth: 12 feet
Time Period: 0.25 hour
Upstream Signals:
Link Index: #42
Dist(miles): 0.000
Speed (mph): 0.00
SignalIndex: #3793
Cycle Time: 0 secs
InitVolume: 0 0
Saturation: 0 0
ArrivalType: 0 0
G/C: 0.00 0.00
*** Computation 1: Time for Queue to Clear at Each Upstream Intersection
P: 0.000 0.000
gq1: 0.00 0.00
gq2: 0.00 0.00
gq: 0.00 0.00
*** Computation 2: Time Intersection Blocked Because of Upstream Platoons
alpha: 0.000
beta: 0.000
ta (secs): 0.000
F: 0.000
f: 0.000 0.000
vemax: 0 0
vcg: 0 0
vcmin: 0 0
tp: 0.0 0.0
p: 0.000
*** Computation 3: Platoon Event Periods
pdom/psubo: 0.000/0.000/Unconstrained
*** Computation 4: Conflicting Flows During Each Unblocked Period
InitCnflVol: 0 xxxxxx xxxxxx 0 xxxxxx xxxxxx 1840 2742 468 1789 2726 451
AdjCnflVol: 0 xxxxxx xxxxxx 0 xxxxxx xxxxxx 1840 2742 468 1789 2726 451
UpstreamAdj: 1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
ConflictVol: 0 xxxxxx xxxxxx 0 xxxxxx xxxxxx 1840 2742 468 1789 2726 451
*** Computation 5: Capacity for Subject Movement During Unblocked Period
InitPotCap: 1636 xxxxxx xxxxxx 1636 xxxxxx xxxxxx 48 20 547 74 21 561

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
 Potent Cap.:1636 xxxxxx xxxxxx 1636 xxxxxx xxxxxx 48 20 547 74 21 561
 Peak Hour Delay Signal Warrant Report

 Intersection #3 Saratoga Avenue/Mitzi Drive

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 0 0 2 1 0 0 0 3 0 0 0 0 0 0 0 0 0 0 0 1
 Initial Vol: 0 1305 32 0 1405 0 0 0 0 0 0 0 0 23
 ApproachDel: xxxxxx xxxxxx xxxxxx 11.7

 Approach[westbound][lanes=1][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.1]
 FAIL - Vehicle-hours less than 4 for one lane approach.
 Signal Warrant Rule #2: [approach volume=23]
 FAIL - Approach volume less than 100 for one lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=2765]
 SUCCEED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

 SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #3 Saratoga Avenue/Mitzi Drive

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 0 0 2 1 0 0 0 3 0 0 0 0 0 0 0 0 0 0 0 1
 Initial Vol: 0 1305 32 0 1405 0 0 0 0 0 0 0 0 23

 Major Street Volume: 2742
 Minor Approach Volume: 23
 Minor Approach Volume Threshold: -63 [less than minimum of 100]

 SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Existing PM

Intersection #4: Mitzi Drive/Piper Drive

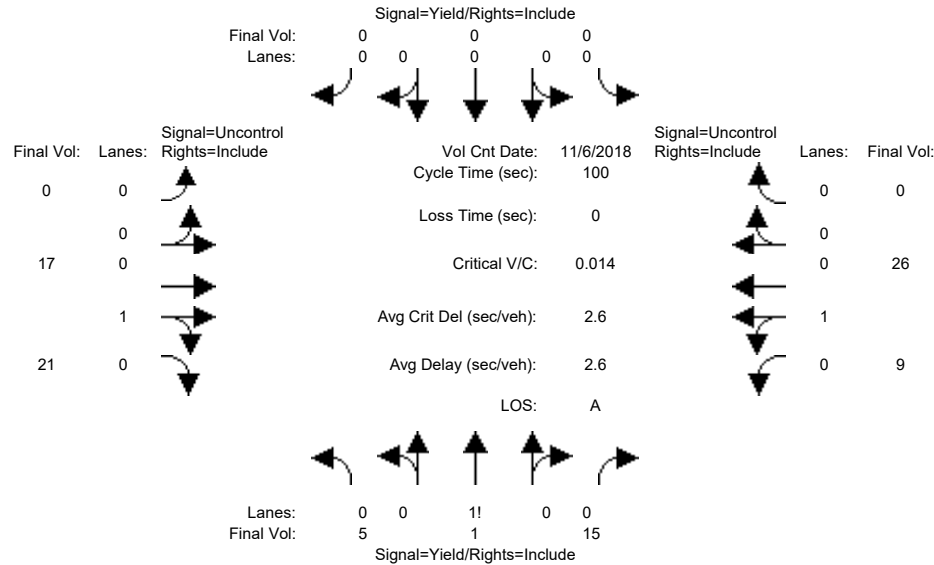


Table with columns for Street Name, Approach, Movement, Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters.

Table for Capacity Module showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Table with columns for HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, and Time Period.

Peak Hour Delay Signal Warrant Report
Intersection #4 Mitzi Drive/Piper Drive
Future Volume Alternative: Peak Hour Warrant NOT Met

Table for Future Volume Alternative showing Approach, Movement, Control, Lanes, Initial Vol, and ApproachDel.

Approach[northbound][lanes=1][control=Yield Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Controller not stop sign.
Signal Warrant Rule #2: [approach volume=21]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=94]
FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.

```

*****
Intersection #4 Mitzi Drive/Piper Drive
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|-----|
Control:   Yield Sign      Yield Sign      Uncontrolled    Uncontrolled
Lanes:     0 0 1 0 0      0 0 0 0 0      0 0 0 1 0      0 1 0 0 0
Initial Vol: 5  1  15      0  0  0  0      0  17  21      9  26  0
-----|-----|-----|-----|-----|
Major Street Volume:          73
Minor Approach Volume:       21
Minor Approach Volume Threshold: 917
-----|-----|-----|-----|

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SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Existing PM

Intersection #5: Mitzi Drive/Ranchero Way

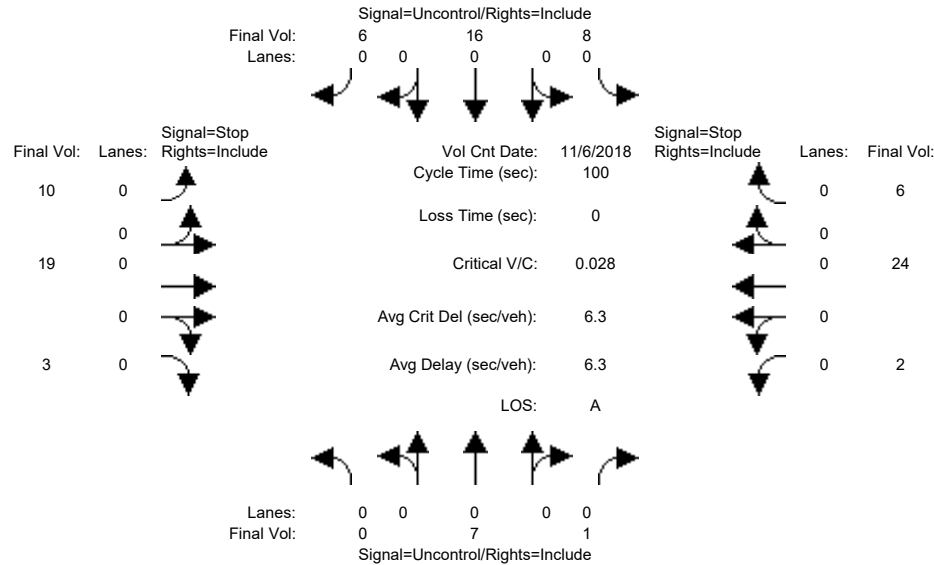


Table with columns for Street Name, Approach, Movement, Volume Module, and Final Volume. Rows include data for Mitzi Drive and Ranchero Way.

Table for Critical Gap Module showing Critical Gap, FollowUp Time, and other parameters for different movements.

Table for Capacity Module showing Conflict Volume, Potent Cap., Move Cap., and Volume/Cap. for various movements.

Table for Level Of Service Module showing LOS by Movement, Shared Cap., Shared Queue, and Shared ConDel for different approaches.

Table with physical characteristics: HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, and Time Period.

Peak Hour Delay Signal Warrant Report
Intersection #5 Mitzi Drive/Ranchero Way
Future Volume Alternative: Peak Hour Warrant NOT Met

Table for Signal Warrant analysis showing Control, Lanes, Initial Vol, and ApproachDel for Eastbound and Westbound approaches.

Signal Warrant Rule #1: [vehicle-hours=0.1]
Signal Warrant Rule #2: [approach volume=32]
Signal Warrant Rule #3: [approach count=4][total volume=102]

Signal Warrant Rule #1: [vehicle-hours=0.1]
Signal Warrant Rule #2: [approach volume=32]
Signal Warrant Rule #3: [approach count=4][total volume=102]

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an
"indicator" of the likelihood of an unsignalized intersection warranting

a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

```

*****
Intersection #5 Mitzi Drive/Ranchero Way
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R        L - T - R        L - T - R        L - T - R
-----|-----|-----|-----|-----|
Control:   Uncontrolled    Uncontrolled    Stop Sign       Stop Sign
Lanes:     0 0 0 1 0            0 0 1! 0 0      0 0 1! 0 0      0 0 1! 0 0
Initial Vol: 0 7 1          8 16 6          10 19 3          2 24 6
-----|-----|-----|-----|-----|
Major Street Volume:          38
Minor Approach Volume:       32
Minor Approach Volume Threshold: 1092
-----|-----|-----|-----|-----|

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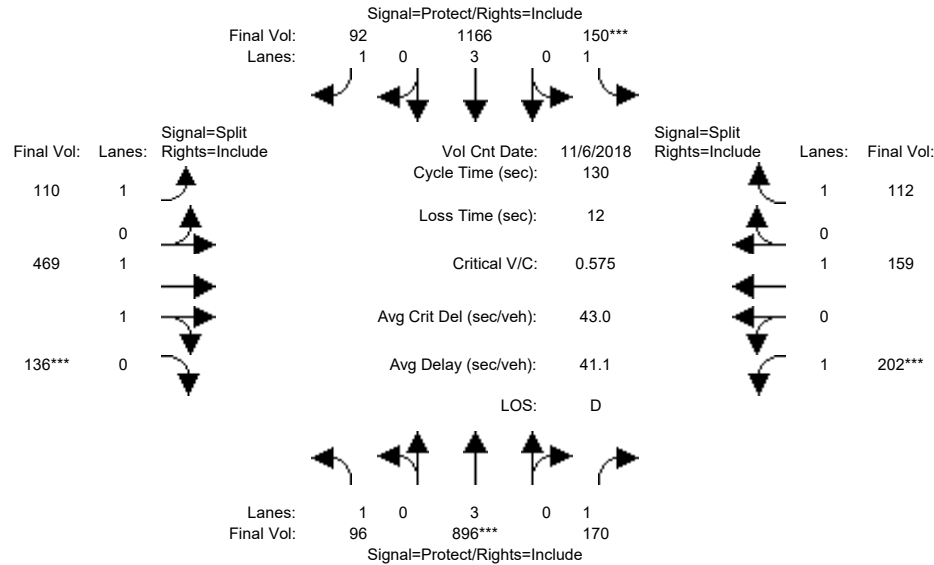
SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
 2000 HCM Operations (Future Volume Alternative)
 Existing PM

Intersection #3793: Saratoga Avenue/Williams Road



| Street Name: | Saratoga Avenue | | | | | | Williams Road | | | | | |
|---------------------------|---|------|------|-------------|------|------|---------------|------|------|------------|------|------|
| | North Bound | | | South Bound | | | East Bound | | | West Bound | | |
| Movement: | L | T | R | L | T | R | L | T | R | L | T | R |
| Min. Green: | 7 | 10 | 10 | 7 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 |
| Y+R: | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Volume Module: | Count Date: 6 Nov 2018 << 4:45 PM - 5:45 PM | | | | | | | | | | | |
| Base Vol: | 96 | 896 | 170 | 150 | 1166 | 92 | 110 | 469 | 136 | 202 | 159 | 112 |
| Growth Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Initial Bse: | 96 | 896 | 170 | 150 | 1166 | 92 | 110 | 469 | 136 | 202 | 159 | 112 |
| Added Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| PasserByVol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Fut: | 96 | 896 | 170 | 150 | 1166 | 92 | 110 | 469 | 136 | 202 | 159 | 112 |
| User Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Volume: | 96 | 896 | 170 | 150 | 1166 | 92 | 110 | 469 | 136 | 202 | 159 | 112 |
| Reduct Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced Vol: | 96 | 896 | 170 | 150 | 1166 | 92 | 110 | 469 | 136 | 202 | 159 | 112 |
| PCE Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| MLF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Final Volume: | 96 | 896 | 170 | 150 | 1166 | 92 | 110 | 469 | 136 | 202 | 159 | 112 |
| Saturation Flow Module: | | | | | | | | | | | | |
| Sat/Lane: | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Adjustment: | 0.92 | 1.00 | 0.92 | 0.92 | 1.00 | 0.92 | 0.92 | 0.98 | 0.95 | 0.92 | 1.00 | 0.92 |
| Lanes: | 1.00 | 3.00 | 1.00 | 1.00 | 3.00 | 1.00 | 1.00 | 1.54 | 0.46 | 1.00 | 1.00 | 1.00 |
| Final Sat.: | 1750 | 5700 | 1750 | 1750 | 5700 | 1750 | 1750 | 2868 | 832 | 1750 | 1900 | 1750 |
| Capacity Analysis Module: | | | | | | | | | | | | |
| Vol/Sat: | 0.05 | 0.16 | 0.10 | 0.09 | 0.20 | 0.05 | 0.06 | 0.16 | 0.16 | 0.12 | 0.08 | 0.06 |
| Crit Moves: | **** | | | | | | | | | | | |
| Green Time: | 11.6 | 35.5 | 35.5 | 19.4 | 43.3 | 43.3 | 37.0 | 37.0 | 37.0 | 26.1 | 26.1 | 26.1 |
| Volume/Cap: | 0.61 | 0.57 | 0.36 | 0.57 | 0.61 | 0.16 | 0.22 | 0.57 | 0.57 | 0.57 | 0.42 | 0.32 |
| Uniform Del: | 57.0 | 40.7 | 38.0 | 51.5 | 36.3 | 30.5 | 35.5 | 39.8 | 39.8 | 46.9 | 45.3 | 44.4 |
| IncrementDel: | 7.1 | 0.5 | 0.5 | 3.1 | 0.6 | 0.1 | 0.2 | 0.8 | 0.8 | 2.3 | 0.7 | 0.5 |
| InitQueueDel: | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Delay Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Delay/Veh: | 64.1 | 41.2 | 38.5 | 54.6 | 36.9 | 30.6 | 35.7 | 40.6 | 40.6 | 49.3 | 46.1 | 44.9 |
| User DelAdj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| AdjDel/Veh: | 64.1 | 41.2 | 38.5 | 54.6 | 36.9 | 30.6 | 35.7 | 40.6 | 40.6 | 49.3 | 46.1 | 44.9 |
| LOS by Move: | E | D | D | D | D | C | D | D | D | D | D | D |
| HCM2k95thQ: | 10 | 19 | 11 | 13 | 24 | 5 | 7 | 20 | 20 | 16 | 11 | 8 |

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Background AM

Intersection #1: Ranchoero Way/Williams Road

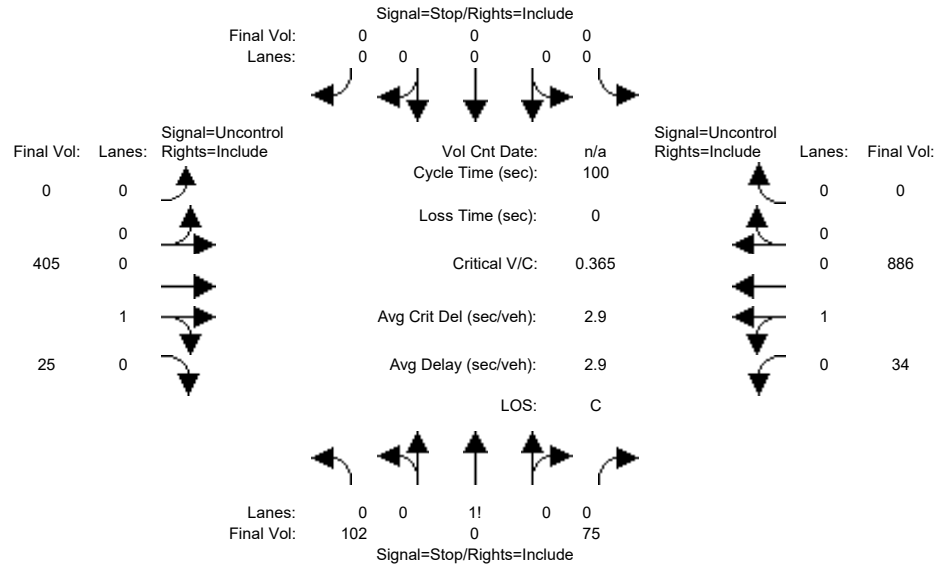


Table containing traffic engineering data: Street Name, Approach, Movement, Volume Module, Critical Gap Module, Capacity Module, Level Of Service Module, and various performance metrics like G/C, P, and alpha.

```

InitPotCap: 163 147 640 117 145 346 1636 xxxxxx xxxxxx 1140 xxxxxx xxxxxx
UpstreamAdj:1.00 1.000 1.000 1.00 1.000 1.000 1.00 x.xxx x.xxx 1.00 x.xxx x.xxx
Potent Cap.: 163 147 640 117 145 346 1636 xxxxxx xxxxxx 1140 xxxxxx xxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Two-Stage Gap Acceptance [Median Type: TWLTL][Median Storage: 1 car]
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Two-Stage Gap Acceptance - Stage One Module:
InitCfltlVol: 418 418 xxxxxx 954 954 xxxxxx xxxxx xxxxxx xxxxxx xxxxxx xxxxxx
UpstreamSat: 0 0 xxxxxx 0 0 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Cnflct Vol: 418 418 xxxxxx 954 954 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
InitPotCap: 669 594 xxxxxx 313 340 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Potent Cap.: 669 594 xxxxxx 313 340 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Move Cap.: 669 594 xxxxxx 304 330 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Two-Stage Gap Acceptance - Stage Two Module:
InitCfltlVol: 954 954 xxxxxx 455 430 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamSat: 0 0 xxxxxx 0 0 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Cnflct Vol: 954 954 xxxxxx 455 430 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
InitPotCap: 377 340 xxxxxx 589 587 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Potent Cap.: 377 340 xxxxxx 589 587 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Move Cap.: 366 330 xxxxxx 520 587 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Peak Hour Delay Signal Warrant Report

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```

*****
Intersection #1 Ranchero Way/Williams Road
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 0 0 1 0 0 0
Initial Vol: 102 0 75 0 0 0 0 0 405 25 34 886 0
ApproachDel: 23.6 xxxxxxx xxxxxxx xxxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=1.2]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=177]
SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=1527]
SUCCEED - Total volume greater than or equal to 650 for intersection
with less than four approaches.
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

```

*****
Intersection #1 Ranchero Way/Williams Road
*****
Future Volume Alternative: Peak Hour Warrant Met
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 0 1 0 0 0
Initial Vol: 102 0 75 0 0 0 0 0 405 25 34 886 0
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Major Street Volume: 1350
Minor Approach Volume: 177
Minor Approach Volume Threshold: 139
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|

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SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Background AM

Intersection #2: Saratoga Avenue/Piper Drive

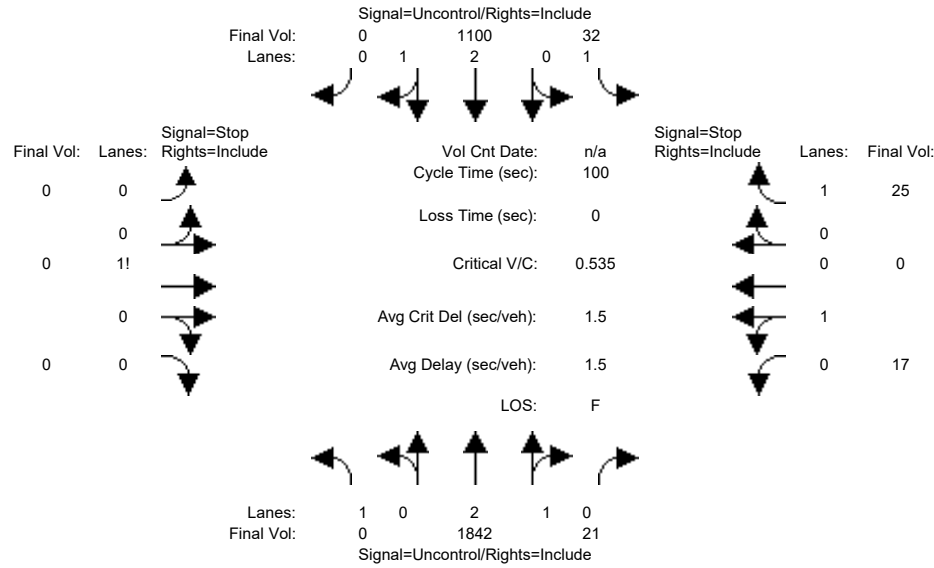


Table with columns for Street Name, Approach, and Movement, detailing volume data for Saratoga Avenue and Piper Drive.

Table for Critical Gap Module showing critical gap and follow-up time values for different movements.

Table for Capacity Module showing conflict volume, potential capacity, and move capacity for various movements.

Table for Level Of Service Module showing control delay, LOS by move, and shared queue data.

Table for Pedestrian and other metrics including HevVeh, Grade, Peds/Hour, and Pedestrian Walk Speed.

Upstream Signals: Link Index, Dist(miles), Speed (mph), SignalIndex, Cycle Time, InitVolume, Saturation, ArrivalType, G/C, and various computation results for queue clearing and platoon blocking.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
 Potent Cap.:1636 xxxxx xxxxx 328 xxxxx xxxxx 53 13 636 34 13 433
 Peak Hour Delay Signal Warrant Report

 Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

| Approach: | North Bound | | | | South Bound | | | | East Bound | | | | West Bound | | | | | | | |
|--------------|--------------|------|----|---|--------------|------|---|---|------------|---|---|---|------------|----|---|----|---|---|---|---|
| Movement: | L | T | R | R | L | T | R | R | L | T | R | R | L | T | R | R | | | | |
| Control: | Uncontrolled | | | | Uncontrolled | | | | Stop Sign | | | | Stop Sign | | | | | | | |
| Lanes: | 1 | 0 | 2 | 1 | 0 | 1 | 0 | 2 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 1 |
| Initial Vol: | 0 | 1842 | 21 | | 32 | 1100 | 0 | | 0 | 0 | 0 | 0 | 0 | 17 | 0 | 25 | | | | |
| ApproachDel: | xxxxxx | | | | xxxxxx | | | | xxxxxx | | | | 93.0 | | | | | | | |

Approach[westbound][lanes=2][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=1.1]
 FAIL - Vehicle-hours less than 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=42]
 FAIL - Approach volume less than 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=3037]
 SUCCEEDED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

| Approach: | North Bound | | | | South Bound | | | | East Bound | | | | West Bound | | | | | | | |
|--------------|--------------|------|----|---|--------------|------|---|---|------------|---|---|---|------------|----|---|----|---|---|---|---|
| Movement: | L | T | R | R | L | T | R | R | L | T | R | R | L | T | R | R | | | | |
| Control: | Uncontrolled | | | | Uncontrolled | | | | Stop Sign | | | | Stop Sign | | | | | | | |
| Lanes: | 1 | 0 | 2 | 1 | 0 | 1 | 0 | 2 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 1 |
| Initial Vol: | 0 | 1842 | 21 | | 32 | 1100 | 0 | | 0 | 0 | 0 | 0 | 0 | 17 | 0 | 25 | | | | |

Major Street Volume: 2995
 Minor Approach Volume: 42
 Minor Approach Volume Threshold: -98 [less than minimum of 150]

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Background AM

Intersection #3: Saratoga Avenue/Mitzi Drive

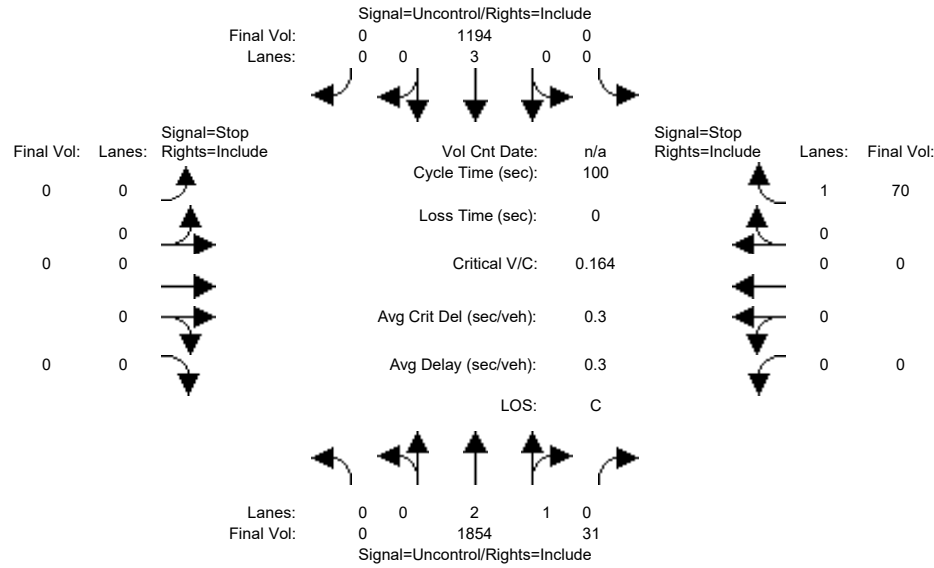


Table with columns for Street Name, Approach, and Movement, detailing traffic volumes for Saratoga Avenue and Mitzi Drive.

Table for Critical Gap Module showing Critical Gap, FollowUpTime, and other metrics.

Table for Capacity Module showing Conflict Vol, Potent Cap, Move Cap, and Volume/Cap.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, and various performance metrics.

Table showing HevVeh, Grade, Peds/Hour, and Pedestrian Walk Speed.

Upstream Signals: Link Index, Dist(miles), Speed(mph), SignalIndex, Cycle Time, InitVolume, Saturation, ArrivalType, G/C, and various computation results for queue clearing and platoon blocking.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
Potent Cap.:1636 xxxxxx xxxxxx 1636 xxxxxx xxxxxx 50 12 607 35 13 427
Peak Hour Delay Signal Warrant Report

Intersection #3 Saratoga Avenue/Mitzi Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

| Approach: | North Bound | South Bound | East Bound | West Bound |
|--------------|--------------|--------------|------------|------------|
| Movement: | L - T - R | L - T - R | L - T - R | L - T - R |
| Control: | Uncontrolled | Uncontrolled | Stop Sign | Stop Sign |
| Lanes: | 0 0 2 1 0 | 0 0 3 0 0 | 0 0 0 0 0 | 0 0 0 0 1 |
| Initial Vol: | 0 1854 31 | 0 1194 0 | 0 0 0 0 0 | 0 0 0 70 |
| ApproachDel: | xxxxxx | xxxxxx | xxxxxx | 15.1 |

Approach[westbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.3]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=70]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=3149]
SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #3 Saratoga Avenue/Mitzi Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

| Approach: | North Bound | South Bound | East Bound | West Bound |
|--------------|--------------|--------------|------------|------------|
| Movement: | L - T - R | L - T - R | L - T - R | L - T - R |
| Control: | Uncontrolled | Uncontrolled | Stop Sign | Stop Sign |
| Lanes: | 0 0 2 1 0 | 0 0 3 0 0 | 0 0 0 0 0 | 0 0 0 0 1 |
| Initial Vol: | 0 1854 31 | 0 1194 0 | 0 0 0 0 0 | 0 0 0 70 |

Major Street Volume: 3079
Minor Approach Volume: 70
Minor Approach Volume Threshold: -103 [less than minimum of 100]

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Background AM

Intersection #4: Mitzi Drive/Piper Drive

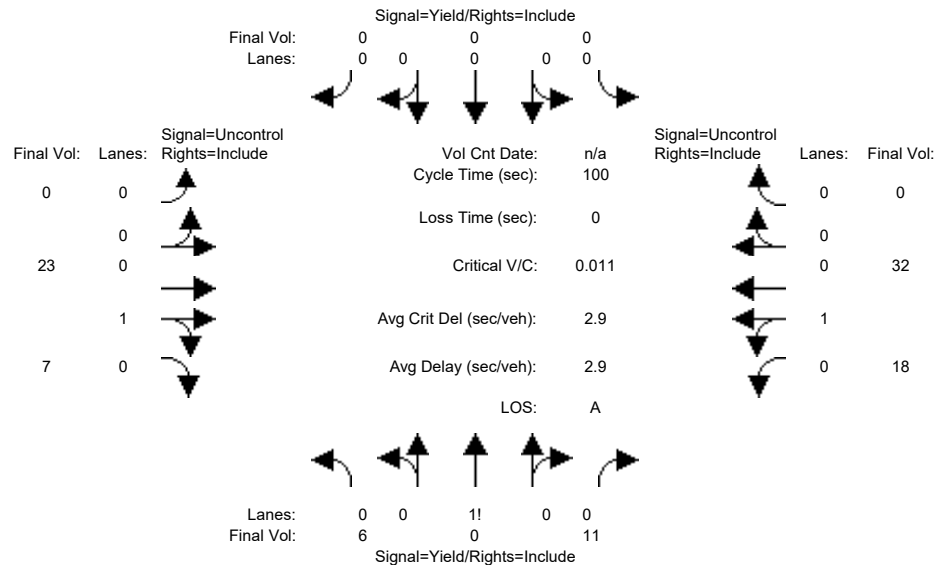


Table with columns for Street Name, Approach, Movement, Volume Module, and Final Volume for Mitzi Drive and Piper Drive.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters.

Table for Capacity Module showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, and other performance metrics.

Table for Pedestrian and Lane Width information, including Peds/Hour, Pedestrian Walk Speed, and Lane Width.

Peak Hour Delay Signal Warrant Report
Intersection #4 Mitzi Drive/Piper Drive
Future Volume Alternative: Peak Hour Warrant NOT Met
Signal Warrant Rule #1: [vehicle-hours=0.0]
Signal Warrant Rule #2: [approach volume=17]
Signal Warrant Rule #3: [approach count=3][total volume=97]

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.

```

*****
Intersection #4 Mitzi Drive/Piper Drive
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R     L - T - R     L - T - R     L - T - R
-----|-----|-----|-----|-----|
Control:    Yield Sign    Yield Sign    Uncontrolled  Uncontrolled
Lanes:      0 0 1 0 0         0 0 0 0 0         0 0 0 1 0         0 1 0 0 0
Initial Vol: 6  0  11         0  0  0  0         0  23  7         18  32  0
-----|-----|-----|-----|-----|
Major Street Volume:      80
Minor Approach Volume:    17
Minor Approach Volume Threshold: 893
-----|-----|-----|-----|-----|

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SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Background AM

Intersection #5: Mitzi Drive/Ranchero Way

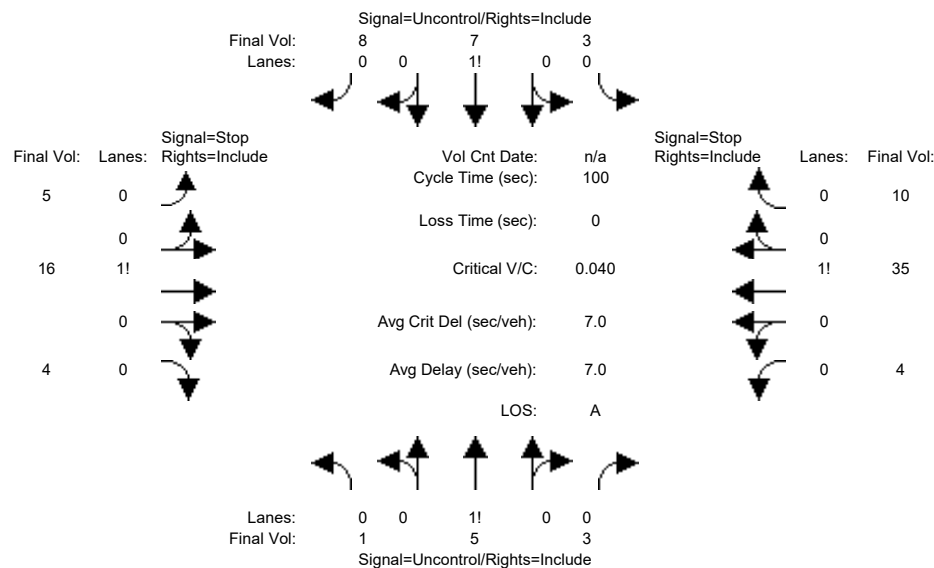


Table with columns for Street Name, Approach, Movement, Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters.

Table for Capacity Module showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.
HevVeh: 0%
Grade: 0%
Peds/Hour: 0
Pedestrian Walk Speed: 4.00 feet/sec
LaneWidth: 12 feet
Time Period: 0.25 hour

Peak Hour Delay Signal Warrant Report
Intersection #5 Mitzi Drive/Ranchero Way

Table for Future Volume Alternative: Peak Hour Warrant NOT Met, showing Control, Lanes, Initial Vol, and ApproachDel for each approach.

Approach[eastbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=25]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=4][total volume=101]
FAIL - Total volume less than 650 for intersection with less than four approaches.
Approach[westbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=49]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=4][total volume=101]
FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting

a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

```

*****
Intersection #5 Mitzi Drive/Ranchero Way
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R     L - T - R     L - T - R     L - T - R
-----|-----|-----|-----|-----|
Control:    Uncontrolled   Uncontrolled   Stop Sign     Stop Sign
Lanes:      0 0 1! 0 0       0 0 1! 0 0       0 0 1! 0 0       0 0 1! 0 0
Initial Vol: 1 5 3       3 7 8         5 16 4         4 4 35 10
-----|-----|-----|-----|-----|
Major Street Volume:          27
Minor Approach Volume:        49
Minor Approach Volume Threshold: 1183
-----|-----|-----|-----|-----|

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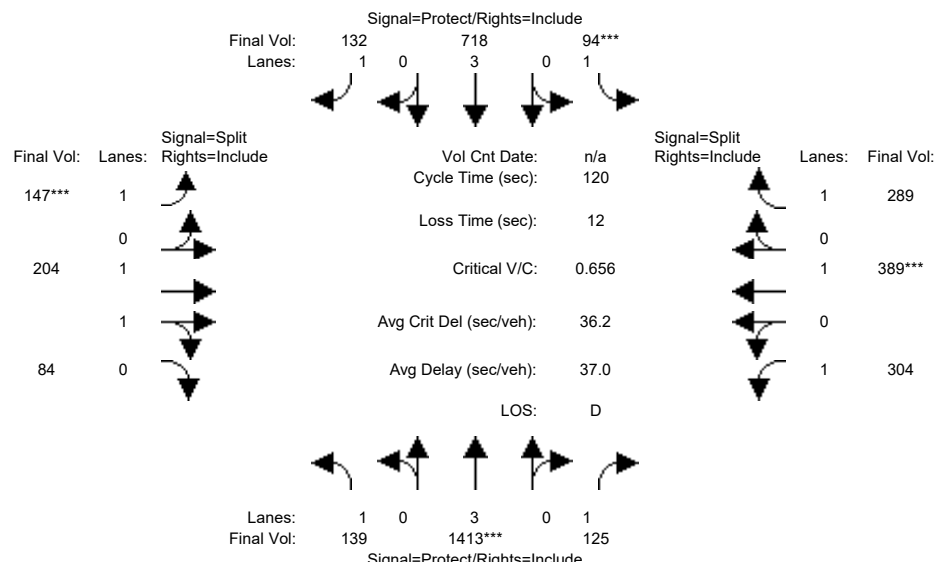
SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
 2000 HCM Operations (Future Volume Alternative)
 Background AM

Intersection #3793: Saratoga Avenue/Williams Road



| Street Name: | Saratoga Avenue | | | | | | Williams Road | | | | | |
|---------------------------|-----------------|------|------|-------------|------|------|---------------|------|------|------------|------|------|
| | North Bound | | | South Bound | | | East Bound | | | West Bound | | |
| Movement: | L | T | R | L | T | R | L | T | R | L | T | R |
| Min. Green: | 7 | 10 | 10 | 7 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 |
| Y+R: | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Volume Module: | | | | | | | | | | | | |
| Base Vol: | 139 | 1413 | 125 | 94 | 718 | 132 | 147 | 204 | 84 | 304 | 389 | 289 |
| Growth Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Initial Bse: | 139 | 1413 | 125 | 94 | 718 | 132 | 147 | 204 | 84 | 304 | 389 | 289 |
| Added Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| PasserByVol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Futr: | 139 | 1413 | 125 | 94 | 718 | 132 | 147 | 204 | 84 | 304 | 389 | 289 |
| User Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Volume: | 139 | 1413 | 125 | 94 | 718 | 132 | 147 | 204 | 84 | 304 | 389 | 289 |
| Reduct Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced Vol: | 139 | 1413 | 125 | 94 | 718 | 132 | 147 | 204 | 84 | 304 | 389 | 289 |
| PCE Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| MLF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Final Volume: | 139 | 1413 | 125 | 94 | 718 | 132 | 147 | 204 | 84 | 304 | 389 | 289 |
| Saturation Flow Module: | | | | | | | | | | | | |
| Sat/Lane: | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Adjustment: | 0.92 | 1.00 | 0.92 | 0.92 | 1.00 | 0.92 | 0.92 | 0.98 | 0.95 | 0.92 | 1.00 | 0.92 |
| Lanes: | 1.00 | 3.00 | 1.00 | 1.00 | 3.00 | 1.00 | 1.00 | 1.40 | 0.60 | 1.00 | 1.00 | 1.00 |
| Final Sat.: | 1750 | 5700 | 1750 | 1750 | 5700 | 1750 | 1750 | 2620 | 1079 | 1750 | 1900 | 1750 |
| Capacity Analysis Module: | | | | | | | | | | | | |
| Vol/Sat: | 0.08 | 0.25 | 0.07 | 0.05 | 0.13 | 0.08 | 0.08 | 0.08 | 0.08 | 0.17 | 0.20 | 0.17 |
| Crit Moves: | *** | | | *** | | | *** | | | *** | | |
| Green Time: | 21.3 | 45.4 | 45.4 | 9.8 | 33.8 | 33.8 | 15.4 | 15.4 | 15.4 | 37.5 | 37.5 | 37.5 |
| Volume/Cap: | 0.45 | 0.66 | 0.19 | 0.66 | 0.45 | 0.27 | 0.66 | 0.61 | 0.61 | 0.56 | 0.66 | 0.53 |
| Uniform Del: | 44.1 | 30.9 | 25.0 | 53.4 | 35.4 | 33.5 | 49.8 | 49.5 | 49.5 | 34.4 | 35.7 | 34.0 |
| IncrementDel: | 1.0 | 0.7 | 0.1 | 10.5 | 0.2 | 0.3 | 6.9 | 2.3 | 2.3 | 1.3 | 2.7 | 1.0 |
| InitQueueDel: | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Delay Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Delay/Veh: | 45.1 | 31.6 | 25.1 | 64.0 | 35.6 | 33.7 | 56.7 | 51.8 | 51.8 | 35.6 | 38.4 | 35.0 |
| User DelAdj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| AdjDel/Veh: | 45.1 | 31.6 | 25.1 | 64.0 | 35.6 | 33.7 | 56.7 | 51.8 | 51.8 | 35.6 | 38.4 | 35.0 |
| LOS by Move: | D | C | C | E | D | C | E | D | D | D | D | C |
| HCM2k95thQ: | 10 | 26 | 7 | 9 | 14 | 8 | 13 | 12 | 12 | 19 | 23 | 18 |

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Background PM

Intersection #1: Ranchoero Way/Williams Road

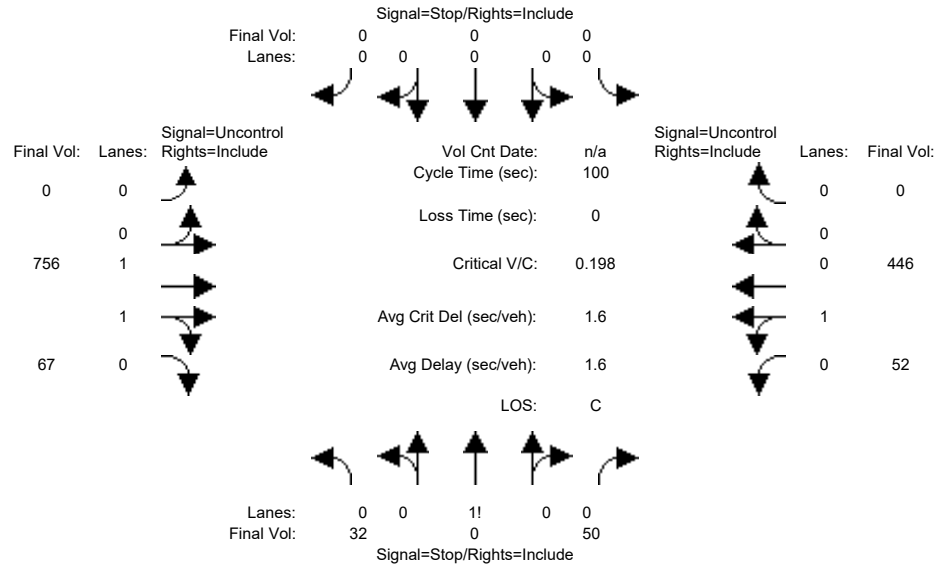


Table with columns for Street Name, Approach, Movement, and Volume Module. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters.

Table for Capacity Module showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. for various movements.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Table with traffic parameters: HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, and Time Period.

Table for Upstream Signals and Computation 1/2/3 showing Link Index, Dist, Speed, Signal Index, Cycle Time, and various delay and saturation metrics.

Table for Computation 4 and 5 showing conflicting flows and capacity for subject movement during unblocked periods.

UpstreamAdj:1.00 1.000 1.000 1.00 1.000 1.000 1.00 x.xxx x.xxx 1.00 x.xxx x.xxx
 Potent Cap.: 170 154 645 250 147 617 1636 xxxxx xxxxx 816 xxxxx xxxxx
 Peak Hour Delay Signal Warrant Report

 Intersection #1 Ranchero Way/Williams Road

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
 Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 1 1 0 0 1 0 0 0
 Initial Vol: 32 0 50 0 0 0 0 0 756 67 52 446 0
 ApproachDel: 21.6 xxxxxx xxxxxx xxxxxx

 Approach[northbound][lanes=1][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.5]
 FAIL - Vehicle-hours less than 4 for one lane approach.
 Signal Warrant Rule #2: [approach volume=82]
 FAIL - Approach volume less than 100 for one lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=1403]
 SUCCEED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

 SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #1 Ranchero Way/Williams Road

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
 Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 1 1 0 0 1 0 0 0
 Initial Vol: 32 0 50 0 0 0 0 0 756 67 52 446 0

 Major Street Volume: 1321
 Minor Approach Volume: 82
 Minor Approach Volume Threshold: 189

 SIGNAL WARRANT DISCLAIMER
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 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

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 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Background PM

Intersection #2: Saratoga Avenue/Piper Drive

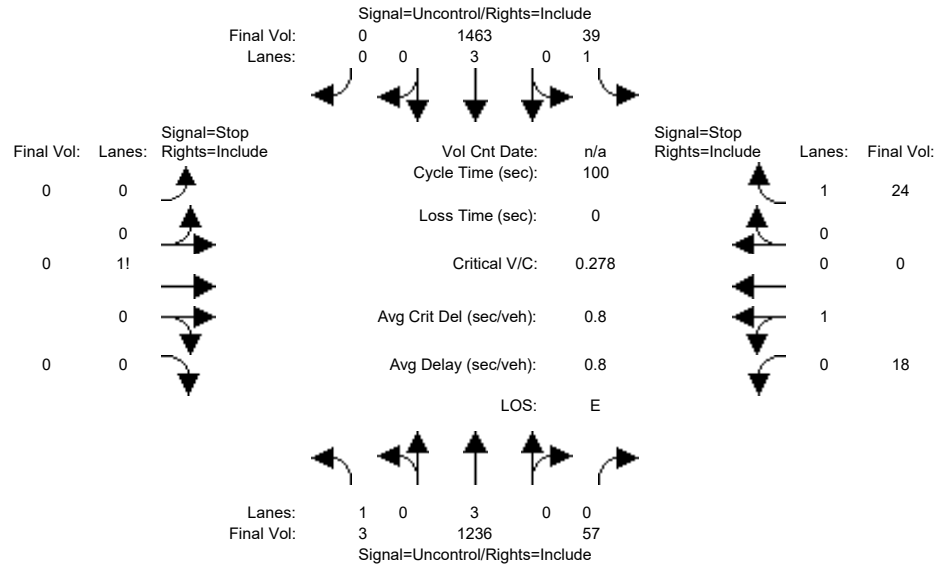


Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters for different movements.

Table for Capacity Module showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. for various movements.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.
HevVeh: 0%
Grade: 0%
Peds/Hour: 0
Pedestrian Walk Speed: 4.00 feet/sec
LaneWidth: 12 feet

Upstream Signals:
Link Index: #42
Dist(miles): 0.000
Speed (mph): 0.00
SignalIndex: #3793
Cycle Time: 0 secs
InitVolume: 0 0
Saturation: 0 0
ArrivalType: 0 0
G/C: 0.00 0.00
*** Computation 1: Time for Queue to Clear at Each Upstream Intersection
P: 0.000 0.000
gq1: 0.00 0.00
gq2: 0.00 0.00
gq: 0.00 0.00
*** Computation 2: Time Intersection Blocked Because of Upstream Platoons
alpha: 0.000
beta: 0.000
ta (secs): 0.000
F: 0.000
F: 0.000 0.000
vcmmax: 0 0
vcg: 0 0
vcmin: 0 0
tp: 0.0 0.0
p: 0.000
*** Computation 3: Platoon Event Periods
pdom/psubo: 0.000/0.000/Unconstrained
*** Computation 4: Conflicting Flows During Each Unblocked Period
InitCnflVol:1463 xxxxxx xxxxxx 1293 xxxxxx xxxxxx 1959 2840 488 1836 2812 441
AdjCnflVol: 1463 xxxxxx xxxxxx 1293 xxxxxx xxxxxx 1959 2840 488 1836 2812 441
UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
ConflictVol:1463 xxxxxx xxxxxx 1293 xxxxxx xxxxxx 1959 2840 488 1836 2812 441
*** Computation 5: Capacity for Subject Movement During Unblocked Period
InitPotCap: 468 xxxxxx xxxxxx 543 xxxxxx xxxxxx 39 18 531 69 18 570

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
 Potent Cap.: 468 xxxxx xxxxx 543 xxxxx xxxxx 39 18 531 69 18 570
 Peak Hour Delay Signal Warrant Report

 Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met
 -----|-----|-----|-----|
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 -----|-----|-----|-----|
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 1 0 2 1 0 1 0 3 0 0 0 0 1 0 0 0 0 1 0 0 1
 Initial Vol: 3 1236 57 39 1463 0 0 0 0 0 18 0 24
 ApproachDel: xxxxxx xxxxxx xxxxxx 41.3
 -----|-----|-----|-----|

Approach[westbound][lanes=2][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.5]
 FAIL - Vehicle-hours less than 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=42]
 FAIL - Approach volume less than 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=2840]
 SUCCEED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met
 -----|-----|-----|-----|
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 -----|-----|-----|-----|
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 1 0 2 1 0 1 0 3 0 0 0 0 1 0 0 0 0 1 0 0 1
 Initial Vol: 3 1236 57 39 1463 0 0 0 0 0 18 0 24
 -----|-----|-----|-----|
 Major Street Volume: 2798
 Minor Approach Volume: 42
 Minor Approach Volume Threshold: -68 [less than minimum of 150]
 -----|-----|-----|-----|

SIGNAL WARRANT DISCLAIMER
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 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

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 the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Background PM

Intersection #3: Saratoga Avenue/Mitzi Drive

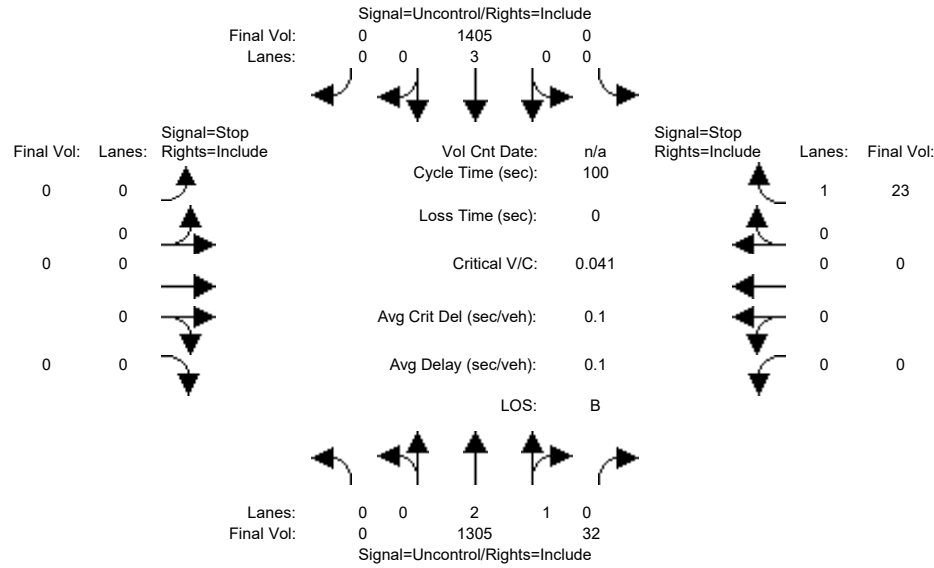


Table containing traffic engineering data: Street Name, Approach, Movement, Volume Module, Critical Gap Module, Capacity Module, Level Of Service Module, and various performance metrics like delay, queue, and saturation.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
 Potent Cap.:1636 xxxxxx xxxxxx 1636 xxxxxx xxxxxx 48 20 547 74 21 561
 Peak Hour Delay Signal Warrant Report

 Intersection #3 Saratoga Avenue/Mitzi Drive

Future Volume Alternative: Peak Hour Warrant NOT Met
 -----|-----|-----|-----|-----|
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 -----|-----|-----|-----|-----|
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 0 0 2 1 0 0 0 3 0 0 0 0 0 0 0 0 0 0 0 1
 Initial Vol: 0 1305 32 0 1405 0 0 0 0 0 0 0 0 0 23
 ApproachDel: xxxxxx xxxxxx xxxxxx 11.7
 -----|-----|-----|-----|-----|

Approach[westbound][lanes=1][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.1]
 FAIL - Vehicle-hours less than 4 for one lane approach.
 Signal Warrant Rule #2: [approach volume=23]
 FAIL - Approach volume less than 100 for one lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=2765]
 SUCCEED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

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 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #3 Saratoga Avenue/Mitzi Drive

Future Volume Alternative: Peak Hour Warrant NOT Met
 -----|-----|-----|-----|-----|
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 -----|-----|-----|-----|-----|
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 0 0 2 1 0 0 0 3 0 0 0 0 0 0 0 0 0 0 0 1
 Initial Vol: 0 1305 32 0 1405 0 0 0 0 0 0 0 0 0 23
 -----|-----|-----|-----|-----|

Major Street Volume: 2742
 Minor Approach Volume: 23
 Minor Approach Volume Threshold: -63 [less than minimum of 100]

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Background PM

Intersection #4: Mitzi Drive/Piper Drive

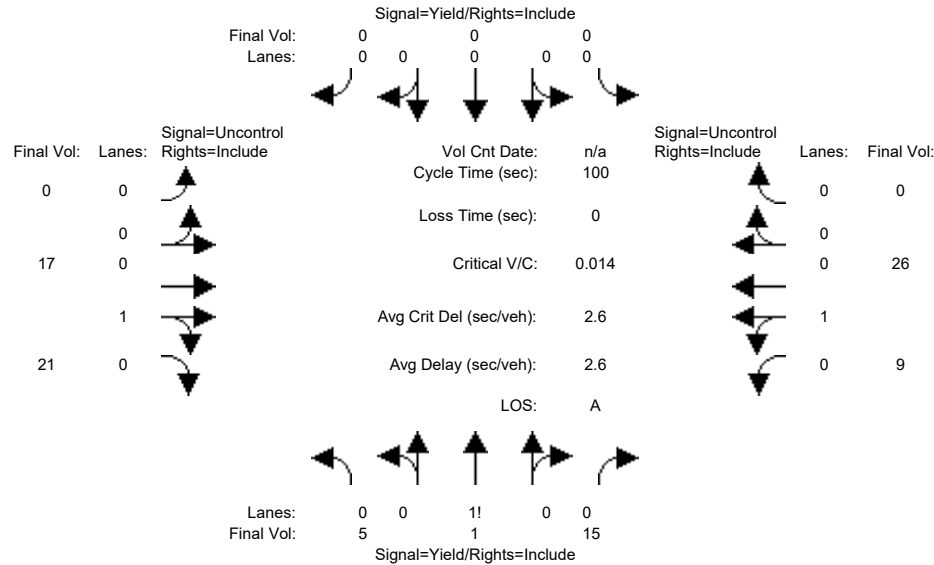


Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Peak Hour Delay Signal Warrant Report
Intersection #4 Mitzi Drive/Piper Drive
Future Volume Alternative: Peak Hour Warrant NOT Met
Signal Warrant Rule #1: [vehicle-hours=0.1]
Signal Warrant Rule #2: [approach volume=21]
Signal Warrant Rule #3: [approach count=3][total volume=94]

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.


```

*****
Intersection #4 Mitzi Drive/Piper Drive
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|-----|
Control:   Yield Sign      Yield Sign      Uncontrolled    Uncontrolled
Lanes:     0 0 1 0 0      0 0 0 0 0      0 0 0 1 0      0 1 0 0 0
Initial Vol: 5  1  15      0  0  0  0      0  17  21      9  26  0
-----|-----|-----|-----|-----|
Major Street Volume: 73
Minor Approach Volume: 21
Minor Approach Volume Threshold: 917
-----|-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER
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Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Background PM

Intersection #5: Mitzi Drive/Ranchero Way

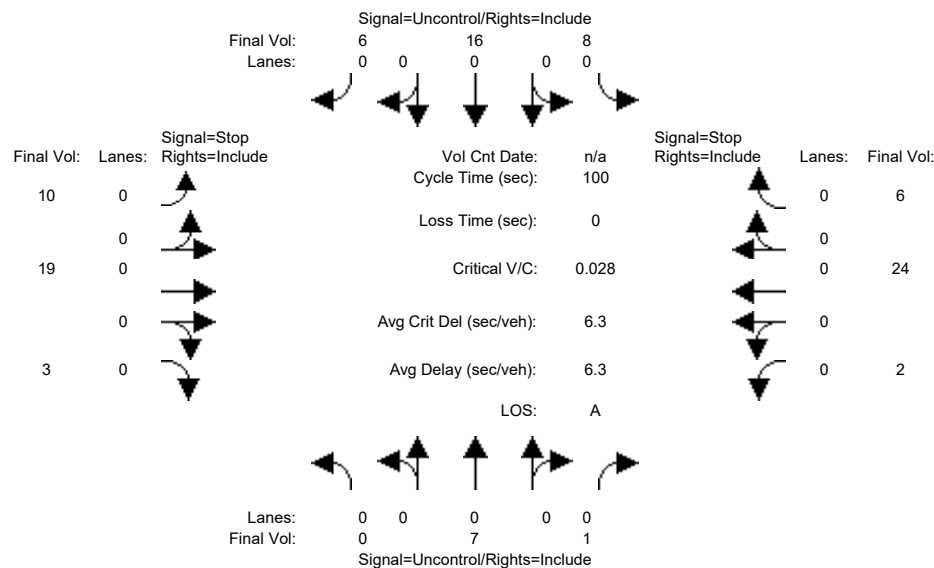


Table with columns for Street Name, Approach, Movement, Volume Module, and Final Volume. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters for different movements.

Table for Capacity Module showing Conflict Vol, Potent Cap, Move Cap, and Volume/Cap for various movements.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Shared Cap, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.
HevVeh: 0%
Grade: 0%
Peds/Hour: 0
Pedestrian Walk Speed: 4.00 feet/sec
LaneWidth: 12 feet
Time Period: 0.25 hour

Peak Hour Delay Signal Warrant Report
Intersection #5 Mitzi Drive/Ranchero Way
Future Volume Alternative: Peak Hour Warrant NOT Met

Table showing Control, Lanes, Initial Vol, and ApproachDel for Eastbound and Westbound approaches.

Approach[eastbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=32]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=4][total volume=102]
FAIL - Total volume less than 650 for intersection with less than four approaches.

Approach[westbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=32]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=4][total volume=102]
FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting

a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Peak Hour Volume Signal Warrant Report [Urban]

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*****
Intersection #5 Mitzi Drive/Ranchero Way
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R     L - T - R     L - T - R     L - T - R
-----|-----|-----|-----|-----|
Control:    Uncontrolled   Uncontrolled   Stop Sign     Stop Sign
Lanes:      0 0 0 1 0         0 0 1! 0 0     0 0 1! 0 0     0 0 1! 0 0
Initial Vol: 0 7 1         8 16 6         10 19 3         2 24 6
-----|-----|-----|-----|-----|
Major Street Volume:      38
Minor Approach Volume:    32
Minor Approach Volume Threshold: 1092
-----|-----|-----|-----|-----|

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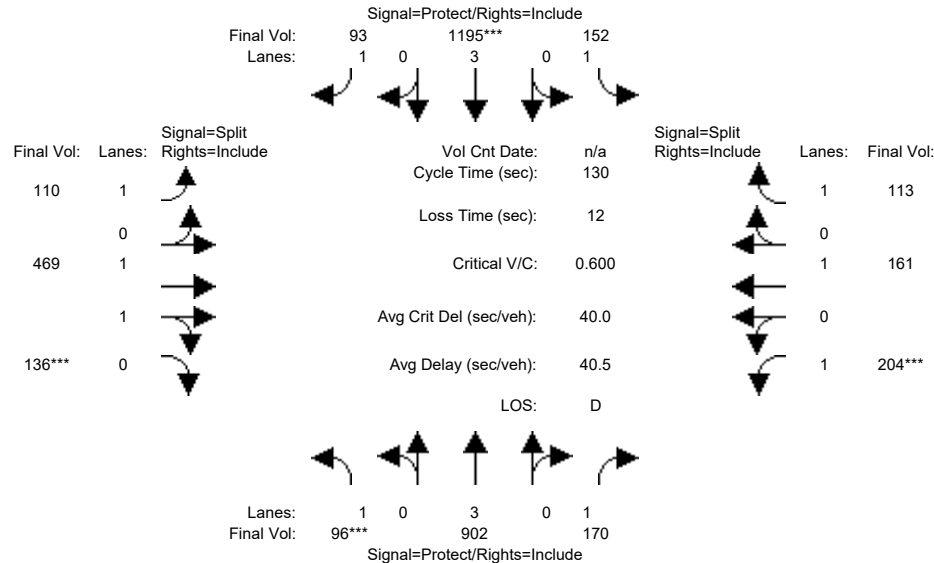
SIGNAL WARRANT DISCLAIMER

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Level Of Service Computation Report
 2000 HCM Operations (Future Volume Alternative)
 Background PM

Intersection #3793: Saratoga Avenue/Williams Road



| Street Name: | Saratoga Avenue | | | | | | Williams Road | | | | | |
|---------------------------|-----------------|------|------|-------------|------|------|---------------|------|------|------------|------|------|
| | North Bound | | | South Bound | | | East Bound | | | West Bound | | |
| Movement: | L | T | R | L | T | R | L | T | R | L | T | R |
| Min. Green: | 7 | 10 | 10 | 7 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 |
| Y+R: | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Volume Module: | 96 | 902 | 170 | 152 | 1195 | 93 | 110 | 469 | 136 | 204 | 161 | 113 |
| Base Vol: | 96 | 902 | 170 | 152 | 1195 | 93 | 110 | 469 | 136 | 204 | 161 | 113 |
| Growth Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Initial Bse: | 96 | 902 | 170 | 152 | 1195 | 93 | 110 | 469 | 136 | 204 | 161 | 113 |
| Added Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| PasserByVol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Fut: | 96 | 902 | 170 | 152 | 1195 | 93 | 110 | 469 | 136 | 204 | 161 | 113 |
| User Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Volume: | 96 | 902 | 170 | 152 | 1195 | 93 | 110 | 469 | 136 | 204 | 161 | 113 |
| Reduct Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced Vol: | 96 | 902 | 170 | 152 | 1195 | 93 | 110 | 469 | 136 | 204 | 161 | 113 |
| PCE Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| MLF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Final Volume: | 96 | 902 | 170 | 152 | 1195 | 93 | 110 | 469 | 136 | 204 | 161 | 113 |
| Saturation Flow Module: | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Sat/Lane: | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Adjustment: | 0.92 | 1.00 | 0.92 | 0.92 | 1.00 | 0.92 | 0.92 | 0.98 | 0.95 | 0.92 | 1.00 | 0.92 |
| Lanes: | 1.00 | 3.00 | 1.00 | 1.00 | 3.00 | 1.00 | 1.00 | 1.54 | 0.46 | 1.00 | 1.00 | 1.00 |
| Final Sat.: | 1750 | 5700 | 1750 | 1750 | 5700 | 1750 | 1750 | 2868 | 832 | 1750 | 1900 | 1750 |
| Capacity Analysis Module: | 0.05 | 0.16 | 0.10 | 0.09 | 0.21 | 0.05 | 0.06 | 0.16 | 0.16 | 0.12 | 0.08 | 0.06 |
| Vol/Sat: | 0.05 | 0.16 | 0.10 | 0.09 | 0.21 | 0.05 | 0.06 | 0.16 | 0.16 | 0.12 | 0.08 | 0.06 |
| Crit Moves: | **** | | | **** | | | **** | | **** | **** | | |
| Green Time: | 11.9 | 37.0 | 37.0 | 20.3 | 45.4 | 45.4 | 35.4 | 35.4 | 35.4 | 25.3 | 25.3 | 25.3 |
| Volume/Cap: | 0.60 | 0.56 | 0.34 | 0.56 | 0.60 | 0.15 | 0.23 | 0.60 | 0.60 | 0.60 | 0.44 | 0.33 |
| Uniform Del: | 56.8 | 39.5 | 36.8 | 50.7 | 34.8 | 29.1 | 36.7 | 41.1 | 41.1 | 47.8 | 46.1 | 45.1 |
| IncrementDel: | 6.2 | 0.4 | 0.4 | 2.5 | 0.5 | 0.1 | 0.2 | 1.0 | 1.0 | 3.0 | 0.8 | 0.6 |
| InitQueueDel: | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Delay Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Delay/Veh: | 63.0 | 39.9 | 37.3 | 53.2 | 35.3 | 29.2 | 37.0 | 42.1 | 42.1 | 50.7 | 46.9 | 45.7 |
| User DelAdj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| AdjDel/Veh: | 63.0 | 39.9 | 37.3 | 53.2 | 35.3 | 29.2 | 37.0 | 42.1 | 42.1 | 50.7 | 46.9 | 45.7 |
| LOS by Move: | E | D | D | D | D | C | D | D | D | D | D | D |
| HCM2k95thQ: | 10 | 19 | 11 | 13 | 24 | 5 | 7 | 20 | 20 | 16 | 11 | 8 |

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Bkgrd+Project AM

Intersection #1: Ranchoero Way/Williams Road

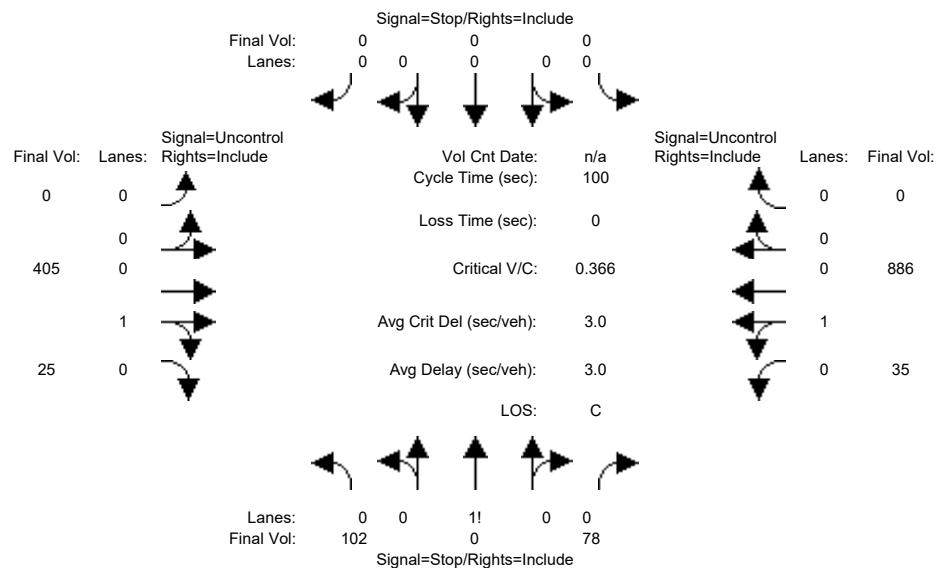


Table containing traffic volume data, LOS module results, and capacity analysis for the intersection. Includes columns for Street Name, Approach, Movement, and various performance metrics like Volume, LOS, and Capacity.

```

InitPotCap: 162 147 640 117 144 346 1636 xxxxxx xxxxxx 1140 xxxxxx xxxxxx
UpstreamAdj:1.00 1.000 1.000 1.00 1.000 1.000 1.00 x.xxx x.xxx 1.00 x.xxx x.xxx
Potent Cap.: 162 147 640 117 144 346 1636 xxxxxx xxxxxx 1140 xxxxxx xxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Two-Stage Gap Acceptance [Median Type: TWLTL][Median Storage: 1 car]
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Two-Stage Gap Acceptance - Stage One Module:
InitCfltlVol: 418 418 xxxxxx 956 956 xxxxxx xxxxx xxxxxx xxxxxx xxxxxx xxxxxx
UpstreamSat: 0 0 xxxxxx 0 0 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Cnflct Vol: 418 418 xxxxxx 956 956 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
InitPotCap: 669 594 xxxxxx 313 339 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Potent Cap.: 669 594 xxxxxx 313 339 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Move Cap.: 669 594 xxxxxx 303 329 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Two-Stage Gap Acceptance - Stage Two Module:
InitCfltlVol: 956 956 xxxxxx 457 430 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamSat: 0 0 xxxxxx 0 0 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Cnflct Vol: 956 956 xxxxxx 457 430 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
InitPotCap: 376 339 xxxxxx 588 587 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Potent Cap.: 376 339 xxxxxx 588 587 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Move Cap.: 365 329 xxxxxx 516 587 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Peak Hour Delay Signal Warrant Report

```

Intersection #1 Ranchero Way/Williams Road

Future Volume Alternative: Peak Hour Warrant NOT Met

| Approach: | North Bound | | | South Bound | | | East Bound | | | West Bound | | |
|--------------|-------------|---|----|-------------|---|---|--------------|-----|----|--------------|-----|---|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Movement: | | | | | | | | | | | | |
| Control: | Stop Sign | | | Stop Sign | | | Uncontrolled | | | Uncontrolled | | |
| Lanes: | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 |
| Initial Vol: | 102 | 0 | 78 | 0 | 0 | 0 | 0 | 405 | 25 | 35 | 886 | 0 |
| ApproachDel: | 23.7 | | | xxxxxxx | | | xxxxxxx | | | xxxxxxx | | |

```

-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=1.2]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=180]
SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=1531]
SUCCEED - Total volume greater than or equal to 650 for intersection
with less than four approaches.
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

```

*****
Intersection #1 Ranchero Way/Williams Road
*****
Future Volume Alternative: Peak Hour Warrant Met
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 1 0 0 0
Initial Vol: 102 0 78 0 0 0 0 0 405 25 35 886 0
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Major Street Volume: 1351
Minor Approach Volume: 180
Minor Approach Volume Threshold: 139
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Level of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Bkgrd+Project AM

Intersection #2: Saratoga Avenue/Piper Drive

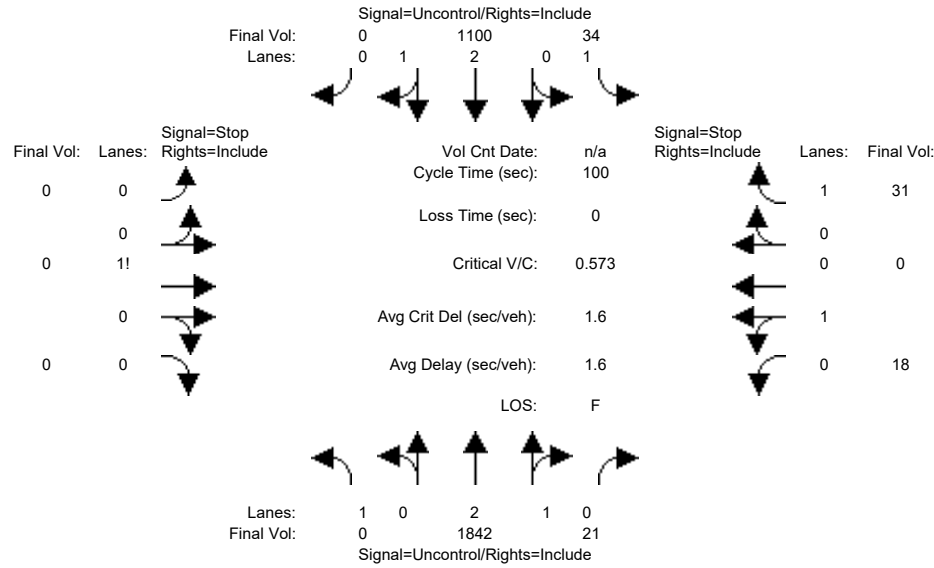


Table containing traffic volume data, LOS calculations, and performance metrics for Saratoga Avenue and Piper Drive. Includes sections for Volume Module, Critical Gap Module, Capacity Module, and Level of Service Module.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
Potent Cap.:1636 xxxxx xxxxx 328 xxxxx xxxxx 53 13 636 34 13 433

Peak Hour Delay Signal Warrant Report

Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

| Approach: | North Bound | South Bound | East Bound | West Bound |
|--------------|--------------|--------------|------------|------------|
| Movement: | L - T - R | L - T - R | L - T - R | L - T - R |
| Control: | Uncontrolled | Uncontrolled | Stop Sign | Stop Sign |
| Lanes: | 1 0 2 1 0 | 1 0 2 1 0 | 0 0 1 0 0 | 0 1 0 0 1 |
| Initial Vol: | 0 1842 21 | 34 1100 0 | 0 0 0 0 | 18 0 31 |
| ApproachDel: | xxxxxx | xxxxxx | xxxxxx | 89.9 |

Approach[westbound][lanes=2][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=1.2]
FAIL - Vehicle-hours less than 5 for two or more lane approach.
Signal Warrant Rule #2: [approach volume=49]
FAIL - Approach volume less than 150 for two or more lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=3046]
SUCCEEDED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

| Approach: | North Bound | South Bound | East Bound | West Bound |
|--------------|--------------|--------------|------------|------------|
| Movement: | L - T - R | L - T - R | L - T - R | L - T - R |
| Control: | Uncontrolled | Uncontrolled | Stop Sign | Stop Sign |
| Lanes: | 1 0 2 1 0 | 1 0 2 1 0 | 0 0 1 0 0 | 0 1 0 0 1 |
| Initial Vol: | 0 1842 21 | 34 1100 0 | 0 0 0 0 | 18 0 31 |

Major Street Volume: 2997
Minor Approach Volume: 49
Minor Approach Volume Threshold: -98 [less than minimum of 150]

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Bkgrd+Project AM

Intersection #3: Saratoga Avenue/Mitzi Drive

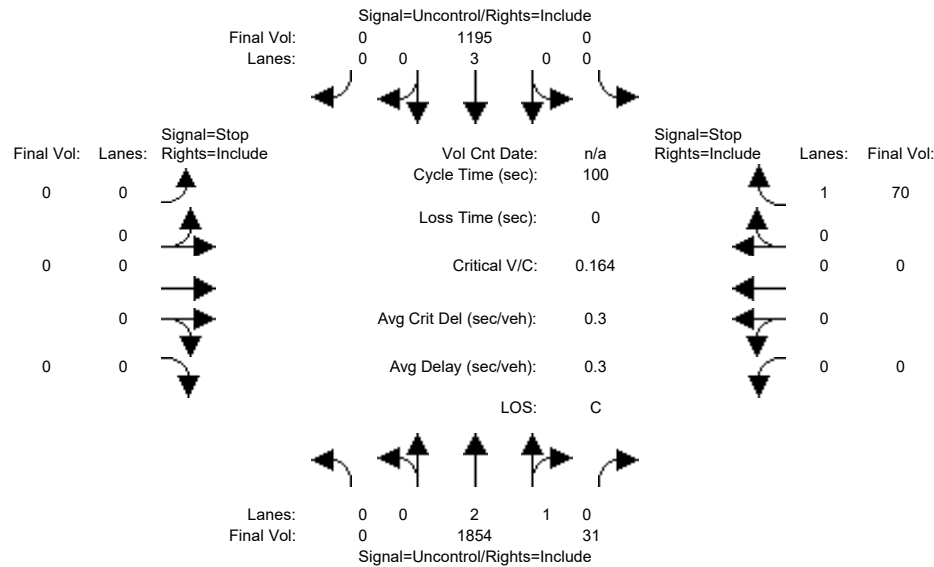


Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Table with columns for Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume, Critical Gap, FollowUpTim, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Table with columns for various performance metrics. Rows include HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, Time Period, Upstream Signals, Link Index, Dist(miles), Speed (mph), SignalIndex, Cycle Time, InitVolume, Saturation, ArrivalType, G/C, Computation 1, Computation 2, Computation 3, Computation 4, Computation 5, InitPotCap.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
Potent Cap.:1636 xxxxx xxxxx 1636 xxxxx xxxxx 50 12 607 35 13 427
Peak Hour Delay Signal Warrant Report

Intersection #3 Saratoga Avenue/Mitzi Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

| Approach: | North Bound | | | South Bound | | | East Bound | | | West Bound | | |
|--------------|--------------|------|----|--------------|------|---|------------|---|---|------------|---|---|
| Movement: | L | T | R | L | T | R | L | T | R | L | T | R |
| Control: | Uncontrolled | | | Uncontrolled | | | Stop Sign | | | Stop Sign | | |
| Lanes: | 0 | 0 | 2 | 1 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 |
| Initial Vol: | 0 | 1854 | 31 | 0 | 1195 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| ApproachDel: | xxxxxx | | | xxxxxx | | | xxxxxx | | | 15.1 | | |

Approach[westbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.3]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=70]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=3150]
SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #3 Saratoga Avenue/Mitzi Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

| Approach: | North Bound | | | South Bound | | | East Bound | | | West Bound | | |
|--------------|--------------|------|----|--------------|------|---|------------|---|---|------------|---|---|
| Movement: | L | T | R | L | T | R | L | T | R | L | T | R |
| Control: | Uncontrolled | | | Uncontrolled | | | Stop Sign | | | Stop Sign | | |
| Lanes: | 0 | 0 | 2 | 1 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 |
| Initial Vol: | 0 | 1854 | 31 | 0 | 1195 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |

Major Street Volume: 3080
Minor Approach Volume: 70
Minor Approach Volume Threshold: -103 [less than minimum of 100]

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Bkgrd+Project AM

Intersection #4: Mitzi Drive/Piper Drive

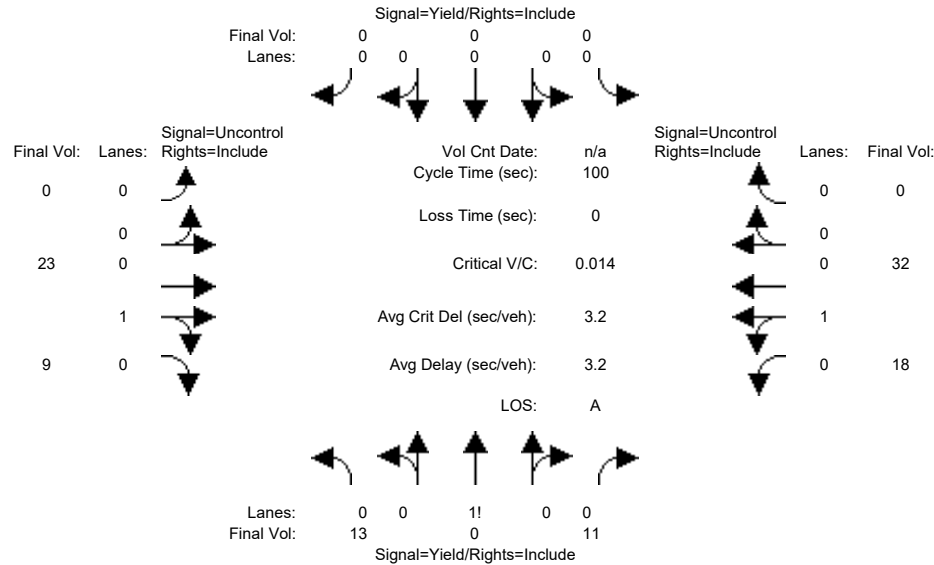


Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module. Data includes various traffic volume and delay metrics.

Table with columns for Street Name, Approach, and Movement. Rows include Capacity Module, Level Of Service Module, and various performance metrics like HevVeh, Grade, and Peds/Hour.

Level Of Service Module:
2Way95thQ:
Control Del:
LOS by Move:
Movement:
Shared Cap.:
SharedQueue:
Shrd ConDel:
Shared LOS:
ApproachDel:
ApproachLOS:

Note: Queue reported is the number of cars per lane.
HevVeh:
Grade:
Peds/Hour:
Pedestrian Walk Speed:
LaneWidth:
Time Period:

Peak Hour Delay Signal Warrant Report
Intersection #4 Mitzi Drive/Piper Drive
Future Volume Alternative: Peak Hour Warrant NOT Met
Approach:
Control:
Lanes:
Initial Vol:
ApproachDel:
Approach[northbound][lanes=1][control=Yield Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
Signal Warrant Rule #2: [approach volume=24]
Signal Warrant Rule #3: [approach count=3][total volume=106]

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

```

*****
Intersection #4 Mitzi Drive/Piper Drive
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|-----|
Control:    Yield Sign      Yield Sign      Uncontrolled    Uncontrolled
Lanes:     0 0 1 0 0      0 0 0 0 0      0 0 0 1 0      0 1 0 0 0
Initial Vol: 13 0 11      0 0 0 0      0 23 9      18 32 0
-----|-----|-----|-----|-----|
Major Street Volume:      82
Minor Approach Volume:    24
Minor Approach Volume Threshold: 886
-----|-----|-----|-----|-----|

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SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Bkgrd+Project AM

Intersection #5: Mitzi Drive/Ranchero Way

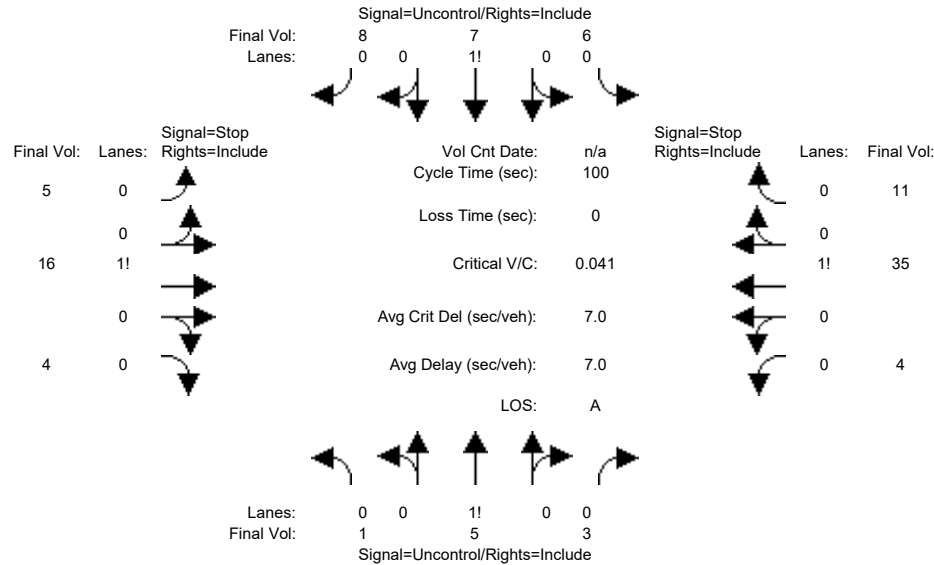


Table with columns: Street Name, Approach, Movement, Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Table with columns: Critical Gap Module, Critical Gap, FollowUpTim.

Table with columns: Capacity Module, Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Table with columns: Level Of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Table with columns: HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, Time Period.

Peak Hour Delay Signal Warrant Report
Intersection #5 Mitzi Drive/Ranchero Way

Future Volume Alternative: Peak Hour Warrant NOT Met

Table with columns: Approach, Movement, Control, Lanes, Initial Vol, ApproachDel.

Approach[eastbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.

Approach[westbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an
"indicator" of the likelihood of an unsignalized intersection warranting

a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

```

*****
Intersection #5 Mitzi Drive/Ranchero Way
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R     L - T - R     L - T - R     L - T - R
-----|-----|-----|-----|-----|
Control:    Uncontrolled   Uncontrolled   Stop Sign     Stop Sign
Lanes:      0 0 1! 0 0     0 0 1! 0 0     0 0 1! 0 0     0 0 1! 0 0
Initial Vol: 1 5 3     6 7 8         5 16 4         4 4 35 11
-----|-----|-----|-----|-----|
Major Street Volume:          30
Minor Approach Volume:        50
Minor Approach Volume Threshold: 1155
-----|-----|-----|-----|-----|

```

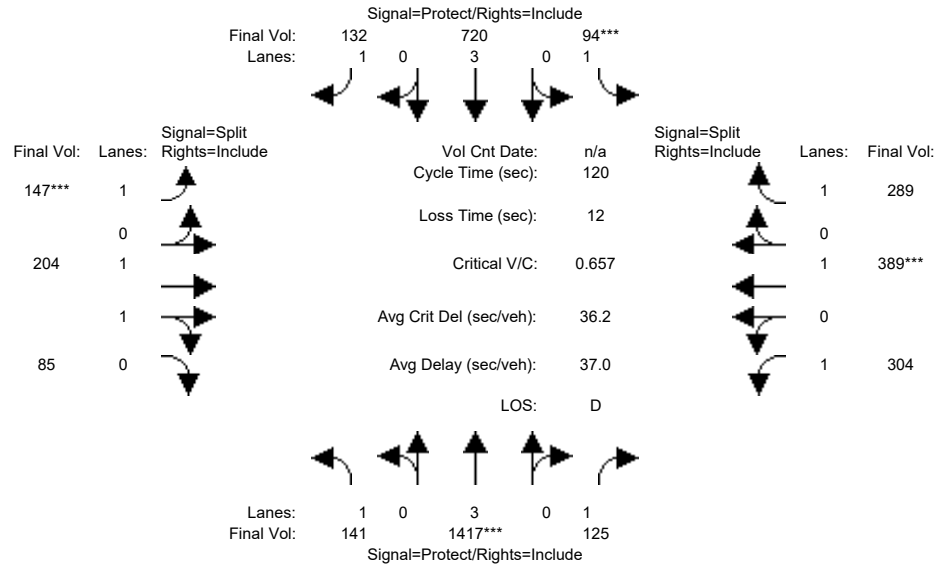
SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Level Of Service Computation Report
 2000 HCM Operations (Future Volume Alternative)
 Bkgrd+Project AM

Intersection #3793: Saratoga Avenue/Williams Road



| Street Name: | Saratoga Avenue | | | | | | Williams Road | | | | | |
|---------------------------|-----------------|------|------|-------------|------|------|---------------|------|------|------------|------|------|
| | North Bound | | | South Bound | | | East Bound | | | West Bound | | |
| Movement: | L | T | R | L | T | R | L | T | R | L | T | R |
| Min. Green: | 7 | 10 | 10 | 7 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 |
| Y+R: | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Volume Module: | | | | | | | | | | | | |
| Base Vol: | 139 | 1413 | 125 | 94 | 718 | 132 | 147 | 204 | 84 | 304 | 389 | 289 |
| Growth Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Initial Bse: | 139 | 1413 | 125 | 94 | 718 | 132 | 147 | 204 | 84 | 304 | 389 | 289 |
| Added Vol: | 2 | 4 | 0 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 0 |
| PasserByVol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Futr: | 141 | 1417 | 125 | 94 | 720 | 132 | 147 | 204 | 85 | 304 | 389 | 289 |
| User Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Volume: | 141 | 1417 | 125 | 94 | 720 | 132 | 147 | 204 | 85 | 304 | 389 | 289 |
| Reduct Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced Vol: | 141 | 1417 | 125 | 94 | 720 | 132 | 147 | 204 | 85 | 304 | 389 | 289 |
| PCE Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| MLF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Final Volume: | 141 | 1417 | 125 | 94 | 720 | 132 | 147 | 204 | 85 | 304 | 389 | 289 |
| Saturation Flow Module: | | | | | | | | | | | | |
| Sat/Lane: | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Adjustment: | 0.92 | 1.00 | 0.92 | 0.92 | 1.00 | 0.92 | 0.92 | 0.98 | 0.95 | 0.92 | 1.00 | 0.92 |
| Lanes: | 1.00 | 3.00 | 1.00 | 1.00 | 3.00 | 1.00 | 1.00 | 1.40 | 0.60 | 1.00 | 1.00 | 1.00 |
| Final Sat.: | 1750 | 5700 | 1750 | 1750 | 5700 | 1750 | 1750 | 2611 | 1088 | 1750 | 1900 | 1750 |
| Capacity Analysis Module: | | | | | | | | | | | | |
| Vol/Sat: | 0.08 | 0.25 | 0.07 | 0.05 | 0.13 | 0.08 | 0.08 | 0.08 | 0.08 | 0.17 | 0.20 | 0.17 |
| Crit Moves: | **** | | | | | | | | | | | |
| Green Time: | 21.5 | 45.4 | 45.4 | 9.8 | 33.7 | 33.7 | 15.3 | 15.3 | 15.3 | 37.4 | 37.4 | 37.4 |
| Volume/Cap: | 0.45 | 0.66 | 0.19 | 0.66 | 0.45 | 0.27 | 0.66 | 0.61 | 0.61 | 0.56 | 0.66 | 0.53 |
| Uniform Del: | 44.0 | 30.8 | 25.0 | 53.5 | 35.5 | 33.5 | 49.8 | 49.5 | 49.5 | 34.4 | 35.7 | 34.0 |
| IncrementDel: | 1.0 | 0.7 | 0.1 | 10.6 | 0.2 | 0.3 | 6.9 | 2.3 | 2.3 | 1.3 | 2.7 | 1.0 |
| InitQueueDel: | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Delay Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Delay/Veh: | 45.0 | 31.6 | 25.1 | 64.1 | 35.7 | 33.8 | 56.7 | 51.8 | 51.8 | 35.7 | 38.4 | 35.0 |
| User DelAdj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| AdjDel/Veh: | 45.0 | 31.6 | 25.1 | 64.1 | 35.7 | 33.8 | 56.7 | 51.8 | 51.8 | 35.7 | 38.4 | 35.0 |
| LOS by Move: | D | C | C | E | D | C | E | D | D | D | D | D |
| HCM2k95thQ: | 10 | 26 | 7 | 10 | 14 | 8 | 13 | 12 | 12 | 19 | 23 | 18 |

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Bkgrd+Project PM

Intersection #1: Ranchoero Way/Williams Road

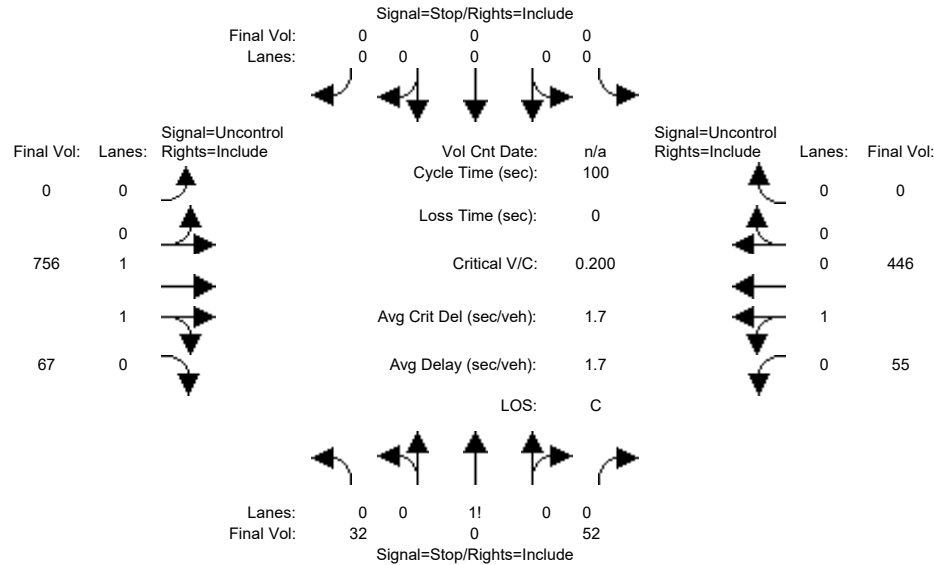


Table with columns for Street Name, Approach, Movement, and Volume Module. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters.

Table for Capacity Module showing Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Table with various performance metrics including HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, Time Period, Upstream Signals, Link Index, Dist(miles), Speed (mph), SignalIndex, Cycle Time, InitVolume, Saturation, ArrivalType, G/C, and various computation results.

UpstreamAdj:1.00 1.000 1.000 1.00 1.000 1.000 1.00 x.xxx x.xxx 1.00 x.xxx x.xxx
 Potent Cap.: 169 153 645 248 146 617 1636 xxxxx xxxxx 816 xxxxx xxxxx
 Peak Hour Delay Signal Warrant Report

 Intersection #1 Ranchero Way/Williams Road

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
 Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 1 1 0 0 1 0 0 0
 Initial Vol: 32 0 52 0 0 0 0 0 0 756 67 55 446 0
 ApproachDel: 21.7 xxxxxx xxxxxx xxxxxx

 Approach[northbound][lanes=1][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.5]
 FAIL - Vehicle-hours less than 4 for one lane approach.
 Signal Warrant Rule #2: [approach volume=84]
 FAIL - Approach volume less than 100 for one lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=1408]
 SUCCEED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

 SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #1 Ranchero Way/Williams Road

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
 Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 1 1 0 0 1 0 0 0
 Initial Vol: 32 0 52 0 0 0 0 0 0 756 67 55 446 0

 Major Street Volume: 1324
 Minor Approach Volume: 84
 Minor Approach Volume Threshold: 188

 SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Bkgrd+Project PM

Intersection #2: Saratoga Avenue/Piper Drive

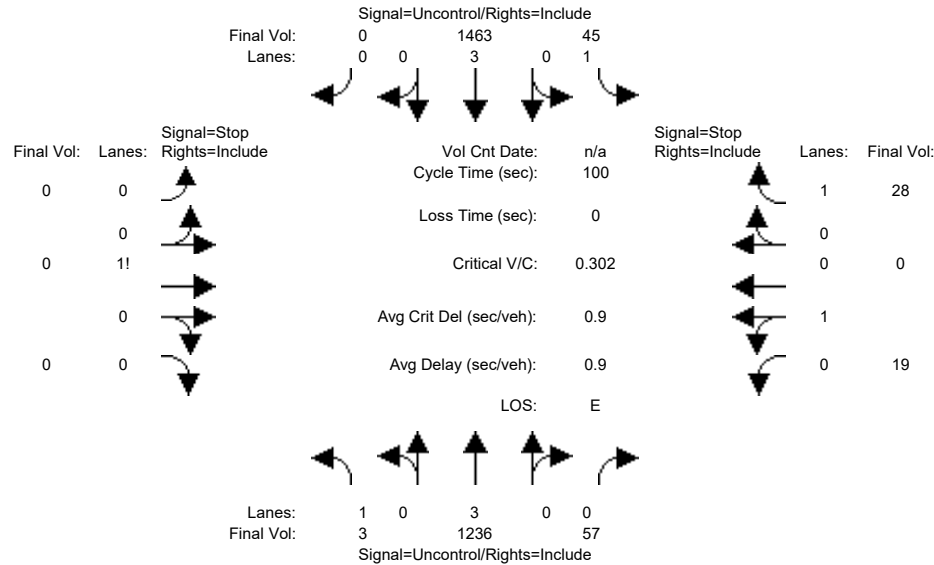


Table with columns for Street Name, Approach, Movement, Volume Module, and Final Volume for Saratoga Avenue and Piper Drive.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters.

Table for Capacity Module showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. ratios.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Table for Pedestrian parameters including HevVeh, Grade, Peds/Hour, and Pedestrian Walk Speed.

Table for Upstream Signals and Computation 1 parameters including Link Index, Dist, Speed, Signal Index, Cycle Time, and various queue and saturation metrics.

Table for Computation 2 parameters including alpha, beta, ta, F, and various queue and saturation metrics.

Table for Computation 3 parameters including pdom/psubo and various queue and saturation metrics.

Table for Computation 4 parameters including InitCnflVol, AdjCnflVol, UpstreamAdj, ConflictVol, and various queue and saturation metrics.

Table for Computation 5 parameters including InitPotCap and various queue and saturation metrics.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
 Potent Cap.: 468 xxxxxx xxxxxx 543 xxxxxx xxxxxx 38 17 531 68 18 570
 Peak Hour Delay Signal Warrant Report

 Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 1 0 2 1 0 1 0 3 0 0 0 0 1 0 0 0 1 0 0 1
 Initial Vol: 3 1236 57 45 1463 0 0 0 0 0 19 0 28
 ApproachDel: xxxxxx xxxxxx xxxxxx 41.4

Approach[westbound][lanes=2][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.5]
 FAIL - Vehicle-hours less than 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=47]
 FAIL - Approach volume less than 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=2851]
 SUCCEEDED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 1 0 2 1 0 1 0 3 0 0 0 0 1 0 0 0 1 0 0 1
 Initial Vol: 3 1236 57 45 1463 0 0 0 0 0 19 0 28

Major Street Volume: 2804
 Minor Approach Volume: 47
 Minor Approach Volume Threshold: -69 [less than minimum of 150]

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Bkgrd+Project PM

Intersection #3: Saratoga Avenue/Mitzi Drive

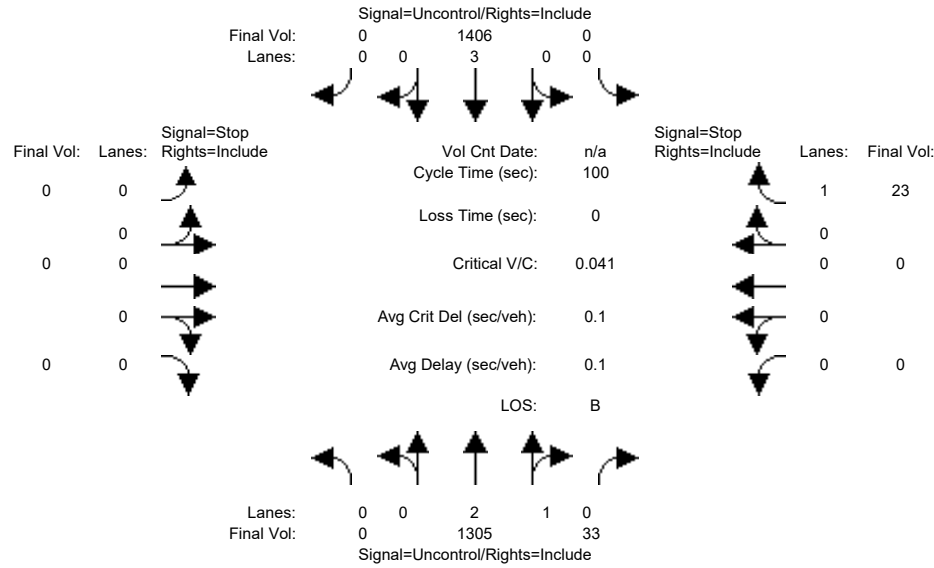


Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Table for Critical Gap Module showing Critical Gap, FollowUpTim, and other timing parameters.

Table for Capacity Module showing Conflict Vol, Potent Cap, Move Cap, and Volume/Cap.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Table for Pedestrian and other metrics including HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, and Time Period.

Table for Upstream Signals and Computation results. Includes Link Index, Dist, Speed, Signal Index, Cycle Time, InitVolume, Saturation, ArrivalType, G/C, and various computation results (1-5) for queue clearing, platoon blocking, and capacity.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
Potent Cap.:1636 xxxxxx xxxxxx 1636 xxxxxx xxxxxx 48 20 547 74 21 561
Peak Hour Delay Signal Warrant Report

Intersection #3 Saratoga Avenue/Mitzi Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Lanes: 0 0 2 1 0 0 0 3 0 0 0 0 0 0 0 0 0 0 0 1
Initial Vol: 0 1305 33 0 1406 0 0 0 0 0 0 0 0 0 23
ApproachDel: xxxxxx xxxxxx xxxxxx 11.7

Approach[westbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=23]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=2767]
SUCCEED - Total volume greater than or equal to 650 for intersection
with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an
"indicator" of the likelihood of an unsignalized intersection warranting
a traffic signal in the future. Intersections that exceed this warrant
are probably more likely to meet one or more of the other volume based
signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
a rigorous and complete traffic signal warrant analysis by the responsible
jurisdiction. Consideration of the other signal warrants, which is beyond
the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #3 Saratoga Avenue/Mitzi Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Lanes: 0 0 2 1 0 0 0 3 0 0 0 0 0 0 0 0 0 0 0 1
Initial Vol: 0 1305 33 0 1406 0 0 0 0 0 0 0 0 0 23

Major Street Volume: 2744
Minor Approach Volume: 23
Minor Approach Volume Threshold: -63 [less than minimum of 100]

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an
"indicator" of the likelihood of an unsignalized intersection warranting
a traffic signal in the future. Intersections that exceed this warrant
are probably more likely to meet one or more of the other volume based
signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
a rigorous and complete traffic signal warrant analysis by the responsible
jurisdiction. Consideration of the other signal warrants, which is beyond
the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Bkgrd+Project PM

Intersection #4: Mitzi Drive/Piper Drive

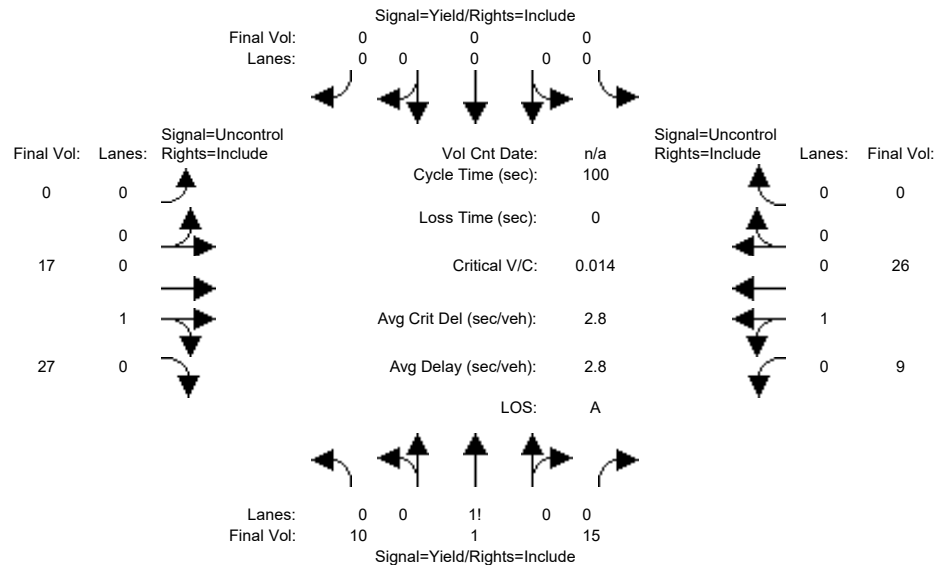


Table with columns for Street Name, Approach, Movement, and Volume Module. It lists traffic volumes for Mitzi Drive (North and South Bound) and Piper Drive (East and West Bound).

Table for Critical Gap Module showing gap times in seconds for various movements and approaches.

Table for Capacity Module showing potential and move capacities for different movements.

Table for Level Of Service Module showing LOS by movement, shared queue, and shared delay/LOS values.

Table with physical characteristics: HevVeh (0%), Grade (0%), Peds/Hour (0), Pedestrian Walk Speed (4.00 feet/sec), LaneWidth (12 feet), and Time Period (0.25 hour).

Peak Hour Delay Signal Warrant Report
Intersection #4 Mitzi Drive/Piper Drive
Future Volume Alternative: Peak Hour Warrant NOT Met

Table showing signal warrant details: Approach, Movement, Control (Yield Sign, Uncontrolled), Lanes, Initial Vol, and ApproachDel.

Signal Warrant Rule #1: [vehicle-hours=0.1] FAIL - Controller not stop sign.
Signal Warrant Rule #2: [approach volume=26] FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=105] FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.

```

*****
Intersection #4 Mitzi Drive/Piper Drive
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|-----|
Control:   Yield Sign      Yield Sign      Uncontrolled    Uncontrolled
Lanes:     0 0 1 0 0      0 0 0 0 0      0 0 0 1 0      0 1 0 0 0
Initial Vol: 10 1 15      0 0 0 0      0 17 27      9 26 0
-----|-----|-----|-----|-----|
Major Street Volume:              79
Minor Approach Volume:            26
Minor Approach Volume Threshold: 896
-----|-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Bkgrd+Project PM

Intersection #5: Mitzi Drive/Ranchero Way

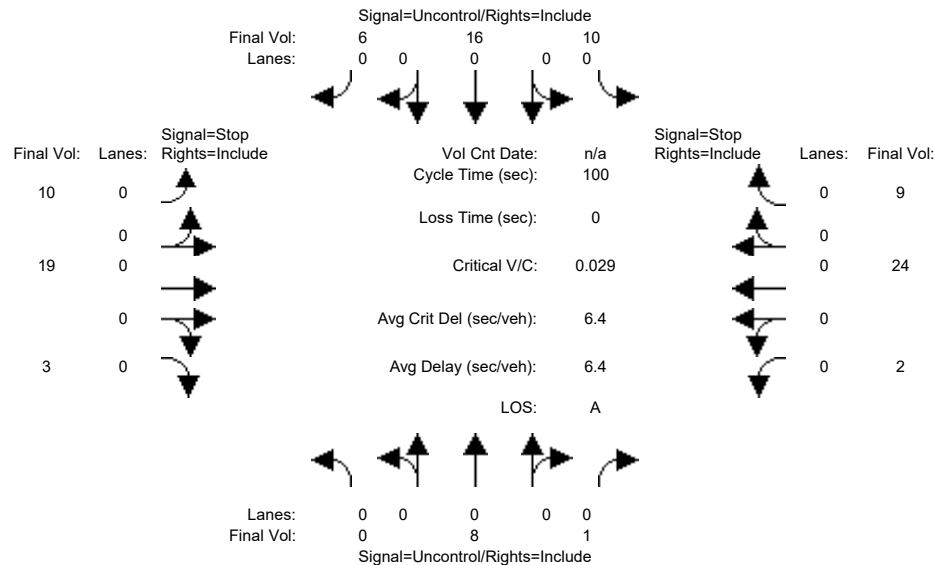


Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Table with columns for Volume/Cap and Level Of Service. Rows include Critical Gap Module, Capacity Module, and Level Of Service Module.

Note: Queue reported is the number of cars per lane.
HevVeh: 0%
Grade: 0%
Peds/Hour: 0
Pedestrian Walk Speed: 4.00 feet/sec
LaneWidth: 12 feet

Peak Hour Delay Signal Warrant Report
Intersection #5 Mitzi Drive/Ranchero Way
Future Volume Alternative: Peak Hour Warrant NOT Met
Approach: North Bound, South Bound, East Bound, West Bound
Control: Uncontrolled, Uncontrolled, Stop Sign, Stop Sign
Signal Warrant Rule #1: [vehicle-hours=0.1]
Signal Warrant Rule #2: [approach volume=32]
Signal Warrant Rule #3: [approach count=4][total volume=108]

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an
"indicator" of the likelihood of an unsignalized intersection warranting

a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

```

*****
Intersection #5 Mitzi Drive/Ranchero Way
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R     L - T - R     L - T - R     L - T - R
-----|-----|-----|-----|-----|
Control:    Uncontrolled   Uncontrolled   Stop Sign     Stop Sign
Lanes:      0 0 0 1 0         0 0 1! 0 0     0 0 1! 0 0     0 0 1! 0 0
Initial Vol: 0 8 1         10 16 6         10 19 3         2 24 9
-----|-----|-----|-----|-----|
Major Street Volume:          41
Minor Approach Volume:        35
Minor Approach Volume Threshold: 1071
-----|-----|-----|-----|-----|

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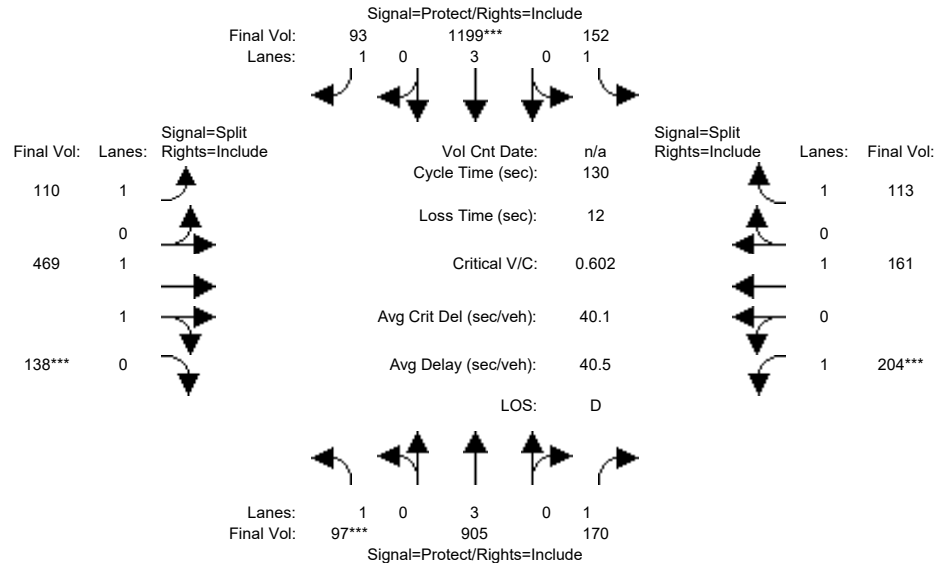
SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
 2000 HCM Operations (Future Volume Alternative)
 Bkgrd+Project PM

Intersection #3793: Saratoga Avenue/Williams Road



| Street Name: | Saratoga Avenue | | | | | | Williams Road | | | | | |
|---------------------------|-----------------|------|------|-------------|------|------|---------------|------|------|------------|------|------|
| | North Bound | | | South Bound | | | East Bound | | | West Bound | | |
| Movement: | L | T | R | L | T | R | L | T | R | L | T | R |
| Min. Green: | 7 | 10 | 10 | 7 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 |
| Y+R: | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Volume Module: | | | | | | | | | | | | |
| Base Vol: | 96 | 902 | 170 | 152 | 1195 | 93 | 110 | 469 | 136 | 204 | 161 | 113 |
| Growth Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Initial Bse: | 96 | 902 | 170 | 152 | 1195 | 93 | 110 | 469 | 136 | 204 | 161 | 113 |
| Added Vol: | 1 | 3 | 0 | 0 | 4 | 0 | 0 | 0 | 2 | 0 | 0 | 0 |
| PasserByVol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Futr: | 97 | 905 | 170 | 152 | 1199 | 93 | 110 | 469 | 138 | 204 | 161 | 113 |
| User Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Volume: | 97 | 905 | 170 | 152 | 1199 | 93 | 110 | 469 | 138 | 204 | 161 | 113 |
| Reduct Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced Vol: | 97 | 905 | 170 | 152 | 1199 | 93 | 110 | 469 | 138 | 204 | 161 | 113 |
| PCE Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| MLF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Final Volume: | 97 | 905 | 170 | 152 | 1199 | 93 | 110 | 469 | 138 | 204 | 161 | 113 |
| Saturation Flow Module: | | | | | | | | | | | | |
| Sat/Lane: | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Adjustment: | 0.92 | 1.00 | 0.92 | 0.92 | 1.00 | 0.92 | 0.92 | 0.98 | 0.95 | 0.92 | 1.00 | 0.92 |
| Lanes: | 1.00 | 3.00 | 1.00 | 1.00 | 3.00 | 1.00 | 1.00 | 1.53 | 0.47 | 1.00 | 1.00 | 1.00 |
| Final Sat.: | 1750 | 5700 | 1750 | 1750 | 5700 | 1750 | 1750 | 2858 | 841 | 1750 | 1900 | 1750 |
| Capacity Analysis Module: | | | | | | | | | | | | |
| Vol/Sat: | 0.06 | 0.16 | 0.10 | 0.09 | 0.21 | 0.05 | 0.06 | 0.16 | 0.16 | 0.12 | 0.08 | 0.06 |
| Crit Moves: | **** | | | **** | | | **** | **** | **** | **** | | |
| Green Time: | 12.0 | 37.1 | 37.1 | 20.3 | 45.4 | 45.4 | 35.4 | 35.4 | 35.4 | 25.2 | 25.2 | 25.2 |
| Volume/Cap: | 0.60 | 0.56 | 0.34 | 0.56 | 0.60 | 0.15 | 0.23 | 0.60 | 0.60 | 0.60 | 0.44 | 0.33 |
| Uniform Del: | 56.7 | 39.5 | 36.8 | 50.7 | 34.8 | 29.1 | 36.7 | 41.1 | 41.1 | 47.8 | 46.2 | 45.2 |
| IncrementDel: | 6.3 | 0.4 | 0.4 | 2.5 | 0.5 | 0.1 | 0.2 | 1.0 | 1.0 | 3.0 | 0.8 | 0.6 |
| InitQueueDel: | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Delay Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Delay/Veh: | 63.0 | 39.9 | 37.2 | 53.2 | 35.4 | 29.2 | 37.0 | 42.2 | 42.2 | 50.9 | 47.0 | 45.8 |
| User DelAdj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| AdjDel/Veh: | 63.0 | 39.9 | 37.2 | 53.2 | 35.4 | 29.2 | 37.0 | 42.2 | 42.2 | 50.9 | 47.0 | 45.8 |
| LOS by Move: | E | D | D | D | D | C | D | D | D | D | D | D |
| HCM2k95thQ: | 10 | 19 | 11 | 13 | 24 | 5 | 7 | 20 | 20 | 16 | 11 | 9 |

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Cumulative AM

Intersection #1: Ranchoero Way/Williams Road

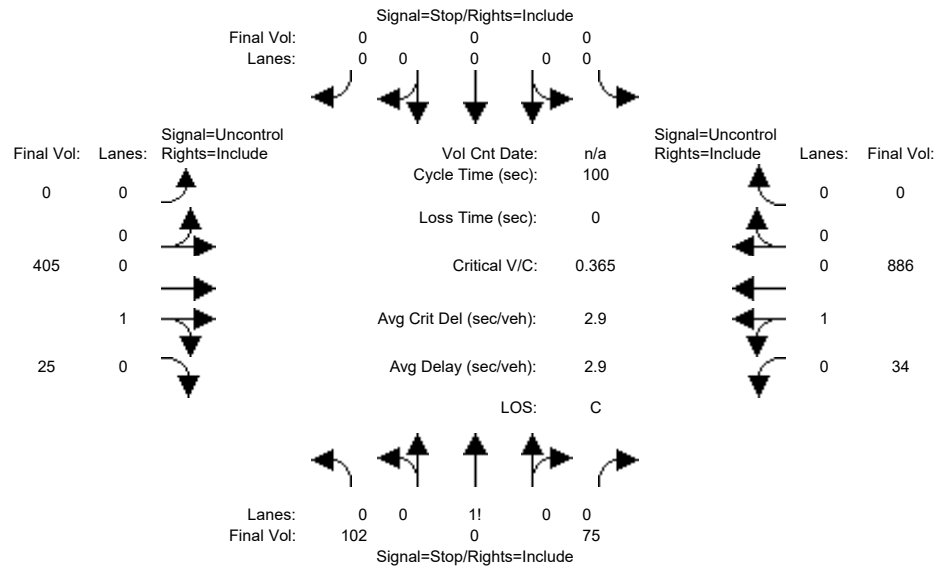


Table containing traffic volume data, LOS module results, and capacity analysis. Key sections include:
- Volume Module: Base Vol, Growth Adj, Initial Bse, etc.
- Critical Gap Module: Critical Gap, FollowUpTim.
- Capacity Module: Cnflct Vol, Potent Cap, Move Cap, Total Cap.
- Level Of Service Module: ZWay95thQ, Control Del, LOS by Move, Shared Cap, etc.
- Note: Queue reported is the number of cars per lane.

```

InitPotCap: 163 147 640 117 145 346 1636 xxxxxx xxxxxx 1140 xxxxxx xxxxxx
UpstreamAdj:1.00 1.000 1.000 1.00 1.000 1.000 1.000 1.00 x.xxx x.xxx 1.00 x.xxx x.xxx
Potent Cap.: 163 147 640 117 145 346 1636 xxxxxx xxxxxx 1140 xxxxxx xxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Two-Stage Gap Acceptance [Median Type: TWLTL][Median Storage: 1 car]
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Two-Stage Gap Acceptance - Stage One Module:
InitCfltlVol: 418 418 xxxxxx 954 954 xxxxxx xxxxx xxxxxx xxxxxx xxxxxx xxxxxx
UpstreamSat: 0 0 xxxxxx 0 0 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Cnflct Vol: 418 418 xxxxxx 954 954 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
InitPotCap: 669 594 xxxxxx 313 340 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Potent Cap.: 669 594 xxxxxx 313 340 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Move Cap.: 669 594 xxxxxx 304 330 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Two-Stage Gap Acceptance - Stage Two Module:
InitCfltlVol: 954 954 xxxxxx 455 430 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamSat: 0 0 xxxxxx 0 0 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Cnflct Vol: 954 954 xxxxxx 455 430 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
InitPotCap: 377 340 xxxxxx 589 587 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
UpstreamAdj:1.00 1.00 xxxxxx 1.00 1.00 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Potent Cap.: 377 340 xxxxxx 589 587 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
Move Cap.: 366 330 xxxxxx 520 587 xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Peak Hour Delay Signal Warrant Report

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*****
Intersection #1 Ranchero Way/Williams Road
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 0 0 1 0 0 0
Initial Vol: 102 0 75 0 0 0 0 0 405 25 34 886 0
ApproachDel: 23.6 xxxxxxx xxxxxxx xxxxxxx
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Approach[northbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=1.2]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=177]
SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
Signal Warrant Rule #3: [approach count=3][total volume=1527]
SUCCEED - Total volume greater than or equal to 650 for intersection
with less than four approaches.

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-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an
"indicator" of the likelihood of an unsignalized intersection warranting
a traffic signal in the future. Intersections that exceed this warrant
are probably more likely to meet one or more of the other volume based
signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
a rigorous and complete traffic signal warrant analysis by the responsible
jurisdiction. Consideration of the other signal warrants, which is beyond
the scope of this software, may yield different results.
Peak Hour Volume Signal Warrant Report [Urban]

```

```

*****
Intersection #1 Ranchero Way/Williams Road
*****
Future Volume Alternative: Peak Hour Warrant Met
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 0 1 0 0 0
Initial Vol: 102 0 75 0 0 0 0 0 405 25 34 886 0
-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
Major Street Volume: 1350
Minor Approach Volume: 177
Minor Approach Volume Threshold: 139

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-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an
"indicator" of the likelihood of an unsignalized intersection warranting
a traffic signal in the future. Intersections that exceed this warrant
are probably more likely to meet one or more of the other volume based
signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
a rigorous and complete traffic signal warrant analysis by the responsible
jurisdiction. Consideration of the other signal warrants, which is beyond
the scope of this software, may yield different results.

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Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Cumulative AM

Intersection #2: Saratoga Avenue/Piper Drive

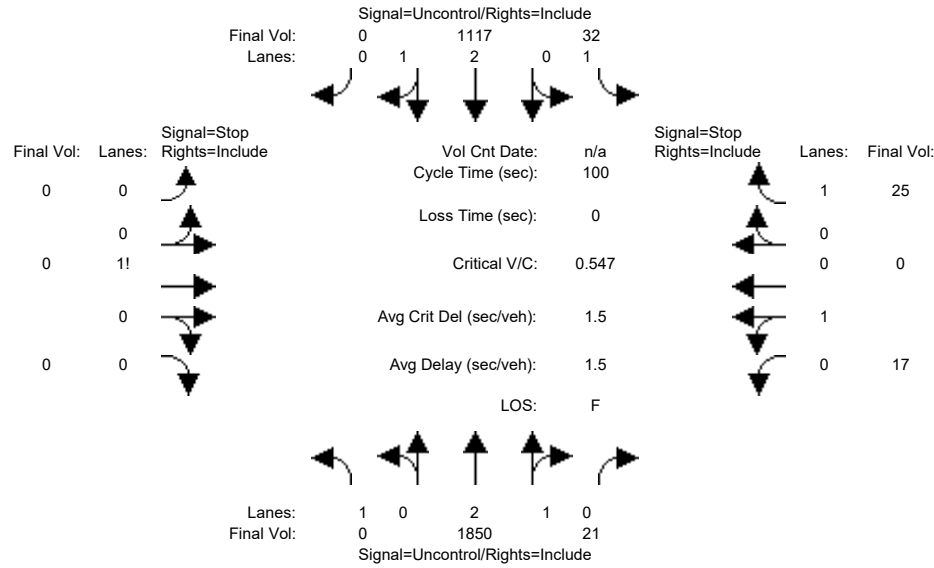


Table containing traffic engineering data: Street Name, Approach, Movement, Volume Module, Critical Gap Module, Capacity Module, Level Of Service Module, and various performance metrics like delay, LOS, and queue lengths.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
 Potent Cap.:1636 xxxxx xxxxx 326 xxxxx xxxxx 51 13 631 34 13 431
 Peak Hour Delay Signal Warrant Report

 Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met
 -----|-----|-----|-----|
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 -----|-----|-----|-----|
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 1 0 2 1 0 1 0 2 1 0 0 0 1 0 0 0 1 0 0 1
 Initial Vol: 0 1850 21 32 1117 0 0 0 0 0 0 17 0 25
 ApproachDel: xxxxxx xxxxxx xxxxxx 95.8
 -----|-----|-----|-----|

Approach[westbound][lanes=2][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=1.1]
 FAIL - Vehicle-hours less than 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=42]
 FAIL - Approach volume less than 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=3062]
 SUCCEED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met
 -----|-----|-----|-----|
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 -----|-----|-----|-----|
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 1 0 2 1 0 1 0 2 1 0 0 0 1 0 0 0 1 0 0 1
 Initial Vol: 0 1850 21 32 1117 0 0 0 0 0 0 17 0 25
 -----|-----|-----|-----|

Major Street Volume: 3020
 Minor Approach Volume: 42
 Minor Approach Volume Threshold: -101 [less than minimum of 150]

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Cumulative AM

Intersection #3: Saratoga Avenue/Mitzi Drive

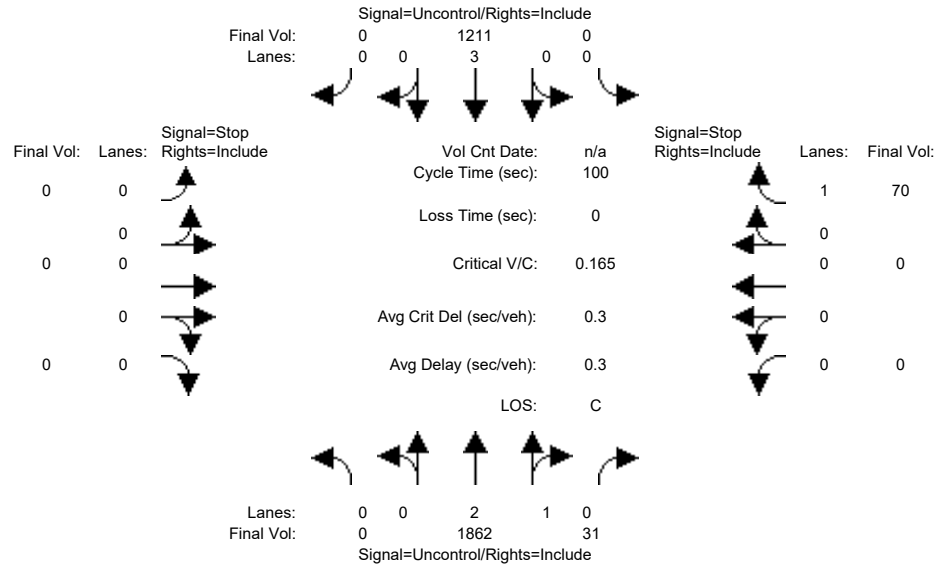


Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Table with columns for Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume, Critical Gap, FollowUpTim, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Table with columns for various performance metrics. Rows include HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, Time Period, Upstream Signals, Link Index, Dist(miles), Speed (mph), SignalIndex, Cycle Time, InitVolume, Saturation, ArrivalType, G/C, Computation 1, Computation 2, Computation 3, Computation 4, Computation 5, InitPotCap.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
Potent Cap.:1636 xxxxxx xxxxxx 1636 xxxxxx xxxxxx 48 12 602 34 12 425

Peak Hour Delay Signal Warrant Report

Intersection #3 Saratoga Avenue/Mitzi Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

| Approach: | North Bound | South Bound | East Bound | West Bound |
|--------------|--------------|--------------|------------|------------|
| Movement: | L - T - R | L - T - R | L - T - R | L - T - R |
| Control: | Uncontrolled | Uncontrolled | Stop Sign | Stop Sign |
| Lanes: | 0 0 2 1 0 | 0 0 3 0 0 | 0 0 0 0 0 | 0 0 0 0 1 |
| Initial Vol: | 0 1862 31 | 0 1211 0 | 0 0 0 0 | 0 0 0 70 |
| ApproachDel: | xxxxxx | xxxxxx | xxxxxx | 15.1 |

Approach[westbound][lanes=1][control=Stop Sign]

Signal Warrant Rule #1: [vehicle-hours=0.3]

FAIL - Vehicle-hours less than 4 for one lane approach.

Signal Warrant Rule #2: [approach volume=70]

FAIL - Approach volume less than 100 for one lane approach.

Signal Warrant Rule #3: [approach count=3][total volume=3174]

SUCCEED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #3 Saratoga Avenue/Mitzi Drive

Future Volume Alternative: Peak Hour Warrant NOT Met

| Approach: | North Bound | South Bound | East Bound | West Bound |
|--------------|--------------|--------------|------------|------------|
| Movement: | L - T - R | L - T - R | L - T - R | L - T - R |
| Control: | Uncontrolled | Uncontrolled | Stop Sign | Stop Sign |
| Lanes: | 0 0 2 1 0 | 0 0 3 0 0 | 0 0 0 0 0 | 0 0 0 0 1 |
| Initial Vol: | 0 1862 31 | 0 1211 0 | 0 0 0 0 | 0 0 0 70 |

Major Street Volume: 3104

Minor Approach Volume: 70

Minor Approach Volume Threshold: -105 [less than minimum of 100]

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Cumulative AM

Intersection #4: Mitzi Drive/Piper Drive

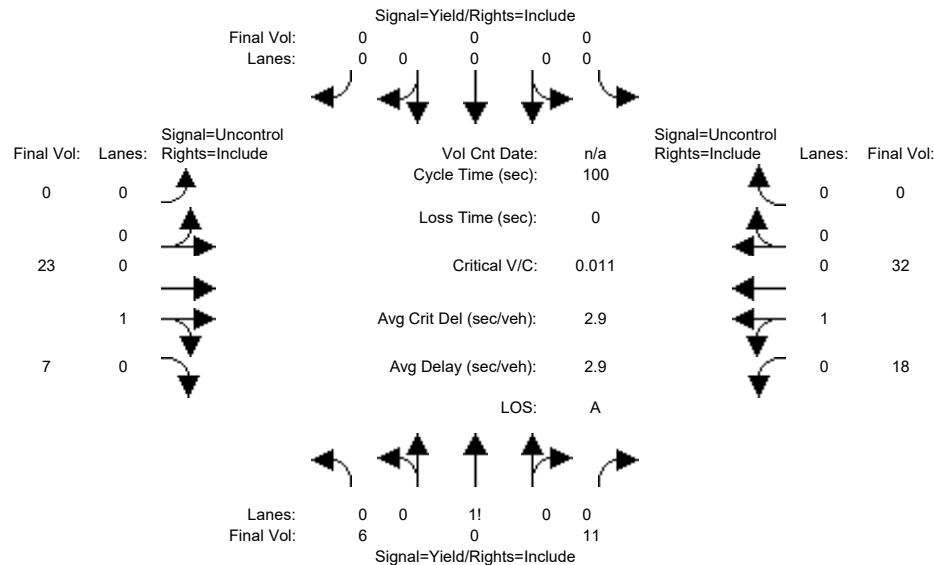


Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Table with columns for Street Name, Approach, and Movement. Rows include Critical Gap Module, Capacity Module, and Level Of Service Module.

Table with columns for Street Name, Approach, and Movement. Rows include Level Of Service Module, Note: Queue reported is the number of cars per lane, and Pedestrian Walk Speed.

Table with columns for Street Name, Approach, and Movement. Rows include Pedestrian Walk Speed, Lane Width, and Time Period.

Peak Hour Delay Signal Warrant Report
Intersection #4 Mitzi Drive/Piper Drive
Future Volume Alternative: Peak Hour Warrant NOT Met
Signal Warrant Rule #1: [vehicle-hours=0.0]
Signal Warrant Rule #2: [approach volume=17]
Signal Warrant Rule #3: [approach count=3][total volume=97]

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.

```

*****
Intersection #4 Mitzi Drive/Piper Drive
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|-----|
Control:   Yield Sign      Yield Sign      Uncontrolled    Uncontrolled
Lanes:     0 0 1 0 0      0 0 0 0 0      0 0 0 1 0      0 1 0 0 0
Initial Vol: 6 0 11      0 0 0 0      0 23 7      18 32 0
-----|-----|-----|-----|-----|
Major Street Volume:      80
Minor Approach Volume:    17
Minor Approach Volume Threshold: 893
-----|-----|-----|-----|-----|

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SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Cumulative AM

Intersection #5: Mitzi Drive/Ranchero Way

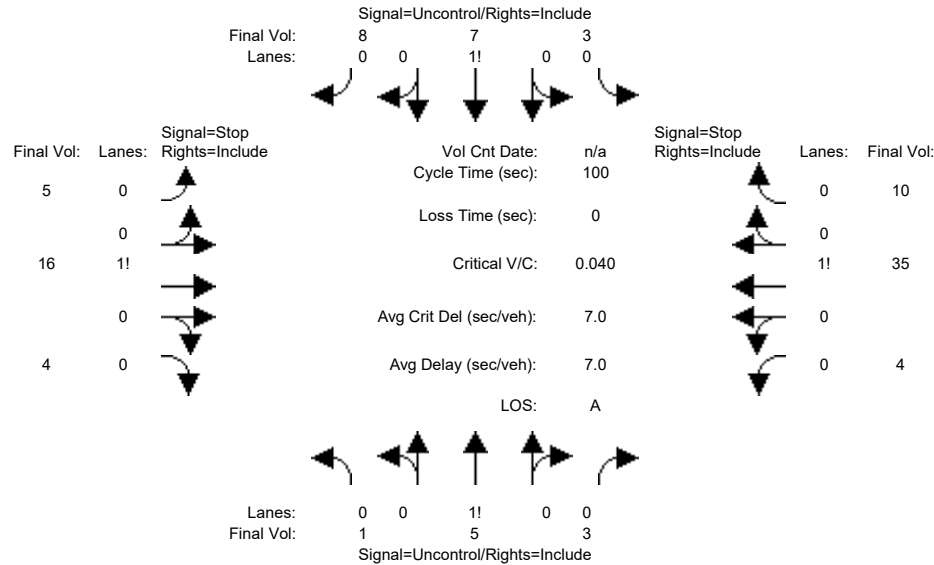


Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Table with columns for Critical Gap Module, Capacity Module, and Level Of Service Module. Rows include Critical Gap, FollowUpTim, Capacity, and LOS metrics.

Table with columns for Level Of Service Module. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Table with columns for various parameters: HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, and Time Period.

Peak Hour Delay Signal Warrant Report
Intersection #5 Mitzi Drive/Ranchero Way
Future Volume Alternative: Peak Hour Warrant NOT Met

Table with columns for Approach, Movement, Control, Lanes, Initial Vol, and ApproachDel. Rows include North Bound, South Bound, East Bound, and West Bound.

Approach[eastbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.

Approach[westbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an
"indicator" of the likelihood of an unsignalized intersection warranting

a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

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*****
Intersection #5 Mitzi Drive/Ranchero Way
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R        L - T - R        L - T - R        L - T - R
-----|-----|-----|-----|-----|
Control:   Uncontrolled    Uncontrolled    Stop Sign       Stop Sign
Lanes:     0 0 1! 0 0          0 0 1! 0 0          0 0 1! 0 0          0 0 1! 0 0
Initial Vol: 1 5 3          3 7 8            5 16 4            4 4 35 10
-----|-----|-----|-----|-----|
Major Street Volume:          27
Minor Approach Volume:       49
Minor Approach Volume Threshold: 1183
-----|-----|-----|-----|-----|

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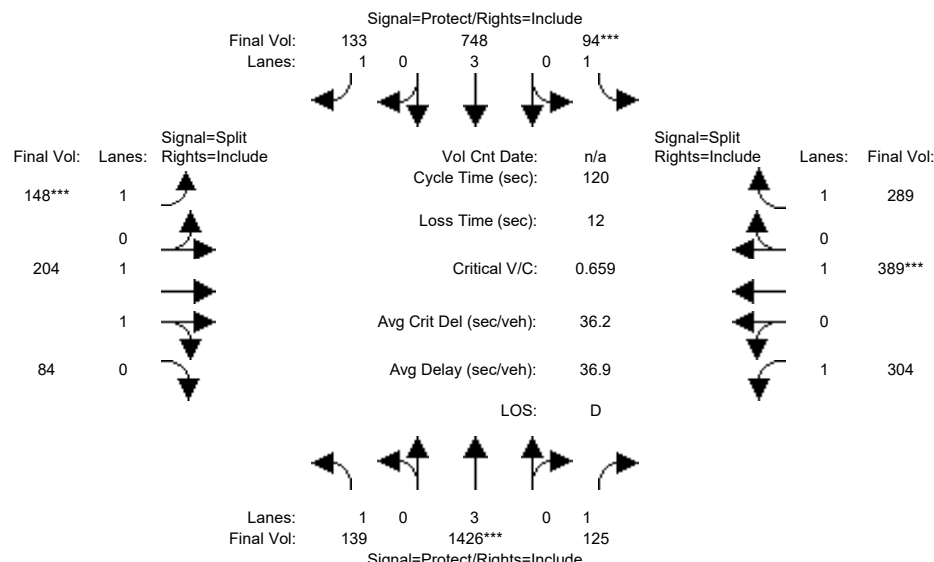
SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
 2000 HCM Operations (Future Volume Alternative)
 Cumulative AM

Intersection #3793: Saratoga Avenue/Williams Road



| Street Name: | Saratoga Avenue | | | | | | Williams Road | | | | | |
|---------------------------|-----------------|------|------|-------------|------|------|---------------|------|------|------------|------|------|
| | North Bound | | | South Bound | | | East Bound | | | West Bound | | |
| Movement: | L | T | R | L | T | R | L | T | R | L | T | R |
| Min. Green: | 7 | 10 | 10 | 7 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 |
| Y+R: | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Volume Module: | | | | | | | | | | | | |
| Base Vol: | 139 | 1426 | 125 | 94 | 748 | 133 | 148 | 204 | 84 | 304 | 389 | 289 |
| Growth Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Initial Bse: | 139 | 1426 | 125 | 94 | 748 | 133 | 148 | 204 | 84 | 304 | 389 | 289 |
| Added Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| PasserByVol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Futr: | 139 | 1426 | 125 | 94 | 748 | 133 | 148 | 204 | 84 | 304 | 389 | 289 |
| User Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Volume: | 139 | 1426 | 125 | 94 | 748 | 133 | 148 | 204 | 84 | 304 | 389 | 289 |
| Reduct Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced Vol: | 139 | 1426 | 125 | 94 | 748 | 133 | 148 | 204 | 84 | 304 | 389 | 289 |
| PCE Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| MLF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| FinalVolume: | 139 | 1426 | 125 | 94 | 748 | 133 | 148 | 204 | 84 | 304 | 389 | 289 |
| Saturation Flow Module: | | | | | | | | | | | | |
| Sat/Lane: | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Adjustment: | 0.92 | 1.00 | 0.92 | 0.92 | 1.00 | 0.92 | 0.92 | 0.98 | 0.95 | 0.92 | 1.00 | 0.92 |
| Lanes: | 1.00 | 3.00 | 1.00 | 1.00 | 3.00 | 1.00 | 1.00 | 1.40 | 0.60 | 1.00 | 1.00 | 1.00 |
| Final Sat.: | 1750 | 5700 | 1750 | 1750 | 5700 | 1750 | 1750 | 2620 | 1079 | 1750 | 1900 | 1750 |
| Capacity Analysis Module: | | | | | | | | | | | | |
| Vol/Sat: | 0.08 | 0.25 | 0.07 | 0.05 | 0.13 | 0.08 | 0.08 | 0.08 | 0.08 | 0.17 | 0.20 | 0.17 |
| Crit Moves: | **** | | | | | | | | | | | |
| Green Time: | 20.9 | 45.5 | 45.5 | 9.8 | 34.5 | 34.5 | 15.4 | 15.4 | 15.4 | 37.3 | 37.3 | 37.3 |
| Volume/Cap: | 0.46 | 0.66 | 0.19 | 0.66 | 0.46 | 0.26 | 0.66 | 0.61 | 0.61 | 0.56 | 0.66 | 0.53 |
| Uniform Del: | 44.5 | 30.8 | 24.9 | 53.5 | 35.1 | 33.0 | 49.8 | 49.4 | 49.4 | 34.5 | 35.9 | 34.2 |
| IncrementDel: | 1.1 | 0.8 | 0.1 | 10.8 | 0.2 | 0.3 | 7.0 | 2.3 | 2.3 | 1.3 | 2.7 | 1.0 |
| InitQueueDel: | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Delay Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Delay/Veh: | 45.6 | 31.6 | 25.0 | 64.3 | 35.3 | 33.3 | 56.8 | 51.7 | 51.7 | 35.8 | 38.6 | 35.2 |
| User DelAdj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| AdjDel/Veh: | 45.6 | 31.6 | 25.0 | 64.3 | 35.3 | 33.3 | 56.8 | 51.7 | 51.7 | 35.8 | 38.6 | 35.2 |
| LOS by Move: | D | C | C | E | D | C | E | D | D | D | D | D |
| HCM2k95thQ: | 10 | 26 | 7 | 10 | 14 | 8 | 13 | 12 | 12 | 19 | 23 | 18 |

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Cumulative PM

Intersection #1: Ranchoero Way/Williams Road

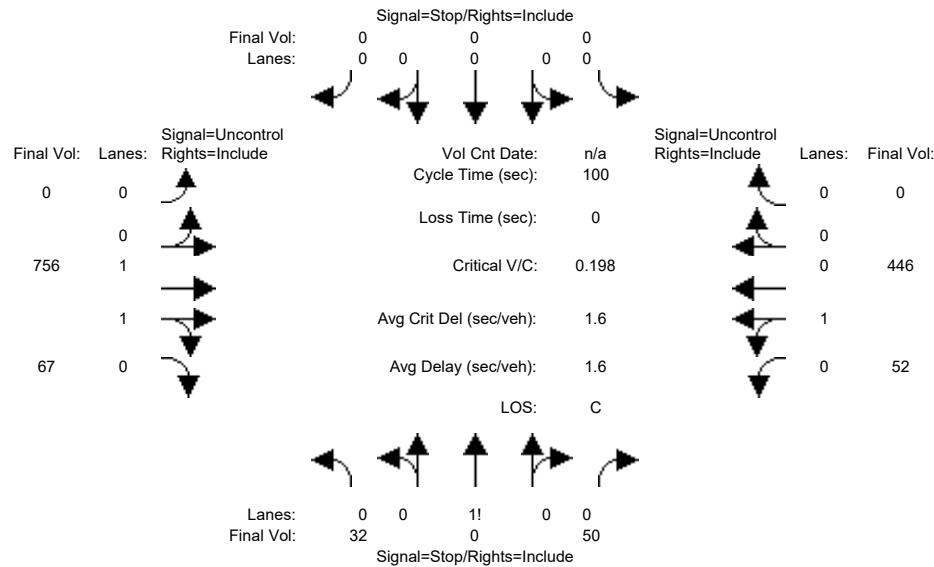


Table with 4 columns: Street Name, Approach, Movement, and Volume. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume for Ranchoero Way and Williams Road.

Table with 4 columns: Critical Gap, FollowUpTim, Capacity, and Volume/Cap. Rows include Critical Gap Module and Capacity Module data.

Table with 4 columns: Level Of Service, Control Del, LOS by Move, Shared Cap, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS. Rows include Level Of Service Module and Note: Queue reported is the number of cars per lane.

Table with 4 columns: HevVeh, Grade, Peds/Hour, Pedestrian Walk Speed, LaneWidth, Time Period, Upstream Signals, Link Index, Dist(miles), Speed (mph), SignalIndex, Cycle Time, InitVolume, Saturation, ArrivalType, G/C, and Computation 1 results.

Table with 4 columns: Computation 2 results (alpha, beta, ta, F, F, vsmax, vcg, vcm, tp, p), Computation 3 results (pdom/psubo), and Computation 4 results (InitCnfVol, AdjCnfVol, UpstreamAdj, ConflictVol, InitPotCap).

Table with 4 columns: Computation 5 results (InitPotCap) and final summary statistics for Ranchoero Way and Williams Road.

UpstreamAdj:1.00 1.000 1.000 1.00 1.000 1.000 1.00 x.xxx x.xxx 1.00 x.xxx x.xxx
 Potent Cap.: 170 154 645 250 147 617 1636 xxxxx xxxxx 816 xxxxx xxxxx
 Peak Hour Delay Signal Warrant Report

 Intersection #1 Ranchero Way/Williams Road

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
 Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 0 1 1 0 0 1 0 0 0
 Initial Vol: 32 0 50 0 0 0 0 0 756 67 52 446 0
 ApproachDel: 21.6 xxxxxx xxxxxx xxxxxx

 Approach[northbound][lanes=1][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.5]
 FAIL - Vehicle-hours less than 4 for one lane approach.
 Signal Warrant Rule #2: [approach volume=82]
 FAIL - Approach volume less than 100 for one lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=1403]
 SUCCEED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

 SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #1 Ranchero Way/Williams Road

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
 Lanes: 0 0 1 0 0 0 0 0 0 0 0 0 0 1 1 0 0 1 0 0 0
 Initial Vol: 32 0 50 0 0 0 0 0 756 67 52 446 0

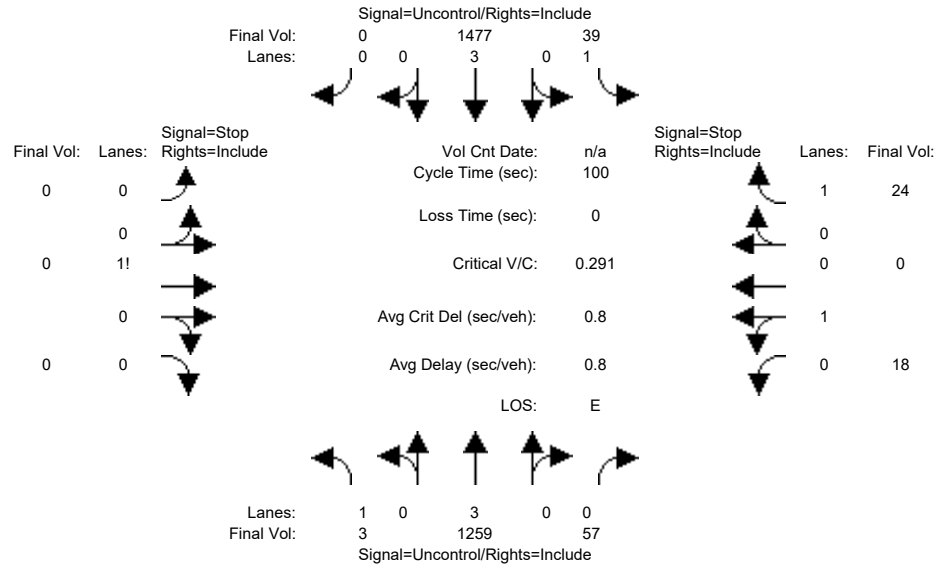
 Major Street Volume: 1321
 Minor Approach Volume: 82
 Minor Approach Volume Threshold: 189

 SIGNAL WARRANT DISCLAIMER
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 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace
 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Level Of Service Computation Report
 2000 HCM Unsignalized (Future Volume Alternative)
 Cumulative PM

Intersection #2: Saratoga Avenue/Piper Drive



| Street Name: | Saratoga Avenue | | | | | Piper Drive | | | | | | |
|---|---------------------------|-------|-------------|---------|-------|-------------|---------|------------|-------|---------|-------|-------|
| | North Bound | | South Bound | | | East Bound | | West Bound | | | | |
| Movement: | L | T | R | L | T | R | L | T | R | L | T | R |
| Volume Module: | | | | | | | | | | | | |
| Base Vol: | 3 | 1259 | 57 | 39 | 1477 | 0 | 0 | 0 | 0 | 18 | 0 | 24 |
| Growth Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Initial Bse: | 3 | 1259 | 57 | 39 | 1477 | 0 | 0 | 0 | 0 | 18 | 0 | 24 |
| Added Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| PasserByVol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Fut: | 3 | 1259 | 57 | 39 | 1477 | 0 | 0 | 0 | 0 | 18 | 0 | 24 |
| User Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Volume: | 3 | 1259 | 57 | 39 | 1477 | 0 | 0 | 0 | 0 | 18 | 0 | 24 |
| Reduct Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| FinalVolume: | 3 | 1259 | 57 | 39 | 1477 | 0 | 0 | 0 | 0 | 18 | 0 | 24 |
| Critical Gap Module: | | | | | | | | | | | | |
| Critical Gp: | 4.1 | xxxx | xxxxx | 4.1 | xxxx | xxxxx | 7.5 | 6.5 | 6.9 | 6.8 | 6.5 | 6.9 |
| FollowUpTim: | 2.2 | xxxx | xxxxx | 2.2 | xxxx | xxxxx | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| Capacity Module: | | | | | | | | | | | | |
| Cnflct Vol: | 1477 | xxxx | xxxxx | 1316 | xxxx | xxxxx | 1981 | 2877 | 492 | 1864 | 2849 | 448 |
| Potent Cap.: | 462 | xxxx | xxxxx | 532 | xxxx | xxxxx | 37 | 17 | 528 | 66 | 17 | 564 |
| Move Cap.: | 462 | xxxx | xxxxx | 532 | xxxx | xxxxx | 34 | 15 | 528 | 62 | 16 | 564 |
| Volume/Cap: | 0.01 | xxxx | xxxx | 0.07 | xxxx | xxxx | 0.00 | 0.00 | 0.00 | 0.29 | 0.00 | 0.04 |
| Level Of Service Module: | | | | | | | | | | | | |
| 2Way95thQ: | 0.0 | xxxx | xxxxx | 0.2 | xxxx | xxxxx | xxxx | xxxx | xxxxx | xxxx | xxxx | 0.1 |
| Control Del: | 12.8 | xxxx | xxxxx | 12.3 | xxxx | xxxxx | xxxxx | xxxx | xxxxx | xxxxx | xxxx | 11.7 |
| LOS by Move: | B | * | * | B | * | * | * | * | * | * | * | * |
| Movement: | LT | LTR | RT | LT | LTR | RT | LT | LTR | RT | LT | LTR | RT |
| Shared Cap.: | xxxx | xxxx | xxxxx | xxxx | xxxx | xxxxx | xxxx | 0 | xxxxx | 62 | xxxx | xxxxx |
| SharedQueue: | xxxxx | xxxx | xxxxx | xxxxx | xxxx | xxxxx | xxxxx | xxxx | xxxxx | 1.0 | xxxx | xxxxx |
| Shrd ConDel: | xxxxx | xxxx | xxxxx | xxxxx | xxxx | xxxxx | xxxxx | xxxx | xxxxx | 85.4 | xxxx | xxxxx |
| Shared LOS: | * | * | * | * | * | * | * | * | * | F | * | * |
| ApproachDel: | xxxxxx | | | xxxxxx | | | xxxxxx | | | 43.3 | | |
| ApproachLOS: | * | | | * | | | * | | | E | | |
| Note: Queue reported is the number of cars per lane. | | | | | | | | | | | | |
| HevVeh: | 0% | | | 0% | | | 0% | | | 0% | | |
| Grade: | 0% | | | 0% | | | 0% | | | 0% | | |
| Peds/Hour: | 0 | | | 0 | | | 0 | | | 0 | | |
| Pedestrian Walk Speed: | 4.00 feet/sec | | | | | | | | | | | |
| LaneWidth: | 12 feet | | | 12 feet | | | 12 feet | | | 12 feet | | |
| Time Period: | 0.25 hour | | | | | | | | | | | |
| Upstream Signals: | | | | | | | | | | | | |
| Link Index: | #42 | | | | | | | | | | | |
| Dist(miles): | 0.000 | | | | | | | | | | | |
| Speed (mph): | 0.00 | | | | | | | | | | | |
| SignalIndex: | #3793 | | | | | | | | | | | |
| Cycle Time: | 0 secs | | | | | | | | | | | |
| InitVolume: | 0 0 | | | | | | | | | | | |
| Saturation: | 0 0 | | | | | | | | | | | |
| ArrivalType: | 0 0 | | | | | | | | | | | |
| G/C: | 0.00 0.00 | | | | | | | | | | | |
| *** Computation 1: Time for Queue to Clear at Each Upstream Intersection | | | | | | | | | | | | |
| P: | 0.000 0.000 | | | | | | | | | | | |
| gg1: | 0.00 0.00 | | | | | | | | | | | |
| gg2: | 0.00 0.00 | | | | | | | | | | | |
| gg: | 0.00 0.00 | | | | | | | | | | | |
| *** Computation 2: Time Intersection Blocked Because of Upstream Platoons | | | | | | | | | | | | |
| alpha: | 0.000 | | | | | | | | | | | |
| beta: | 0.000 | | | | | | | | | | | |
| ta (secs): | 0.000 | | | | | | | | | | | |
| F: | 0.000 | | | | | | | | | | | |
| F: | 0.000 0.000 | | | | | | | | | | | |
| vcmmax: | 0 0 | | | | | | | | | | | |
| vcmg: | 0 0 | | | | | | | | | | | |
| vcmmin: | 0 0 | | | | | | | | | | | |
| tp: | 0.0 0.0 | | | | | | | | | | | |
| p: | 0.000 | | | | | | | | | | | |
| *** Computation 3: Platoon Event Periods | | | | | | | | | | | | |
| pdom/psubo: | 0.000/0.000/Unconstrained | | | | | | | | | | | |
| *** Computation 4: Conflicting Flows During Each Unblocked Period | | | | | | | | | | | | |
| InitCnflVol: | 1477 | xxxxx | xxxxx | 1316 | xxxxx | xxxxx | 1981 | 2877 | 492 | 1864 | 2849 | 448 |
| AdjCnflVol: | 1477 | xxxxx | xxxxx | 1316 | xxxxx | xxxxx | 1981 | 2877 | 492 | 1864 | 2849 | 448 |
| UpstreamAdj: | 1.00 | x.xxx | x.xxx | 1.00 | x.xxx | x.xxx | 1.00 | 1.000 | 1.000 | 1.00 | 1.000 | 1.000 |
| ConflictVol: | 1477 | xxxxx | xxxxx | 1316 | xxxxx | xxxxx | 1981 | 2877 | 492 | 1864 | 2849 | 448 |
| *** Computation 5: Capacity for Subject Movement During Unblocked Period | | | | | | | | | | | | |
| InitPotCap: | 462 | xxxxx | xxxxx | 532 | xxxxx | xxxxx | 37 | 17 | 528 | 66 | 17 | 564 |

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
 Potent Cap.: 462 xxxxx xxxxx 532 xxxxx xxxxx 37 17 528 66 17 564
 Peak Hour Delay Signal Warrant Report

 Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met
 -----|-----|-----|-----|-----|
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 -----|-----|-----|-----|-----|
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 1 0 2 1 0 1 0 3 0 0 0 0 1 0 0 0 0 1 0 0 1
 Initial Vol: 3 1259 57 39 1477 0 0 0 0 0 18 0 24
 ApproachDel: xxxxxx xxxxxx xxxxxx 43.3
 -----|-----|-----|-----|-----|

Approach[westbound][lanes=2][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.5]
 FAIL - Vehicle-hours less than 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=42]
 FAIL - Approach volume less than 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=2877]
 SUCCEED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an
 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

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 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #2 Saratoga Avenue/Piper Drive

Future Volume Alternative: Peak Hour Warrant NOT Met
 -----|-----|-----|-----|-----|
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 -----|-----|-----|-----|-----|
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 1 0 2 1 0 1 0 3 0 0 0 0 1 0 0 0 0 1 0 0 1
 Initial Vol: 3 1259 57 39 1477 0 0 0 0 0 18 0 24
 -----|-----|-----|-----|-----|
 Major Street Volume: 2835
 Minor Approach Volume: 42
 Minor Approach Volume Threshold: -74 [less than minimum of 150]
 -----|-----|-----|-----|-----|

SIGNAL WARRANT DISCLAIMER
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 "indicator" of the likelihood of an unsignalized intersection warranting
 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

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 a rigorous and complete traffic signal warrant analysis by the responsible
 jurisdiction. Consideration of the other signal warrants, which is beyond
 the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Cumulative PM

Intersection #3: Saratoga Avenue/Mitzi Drive

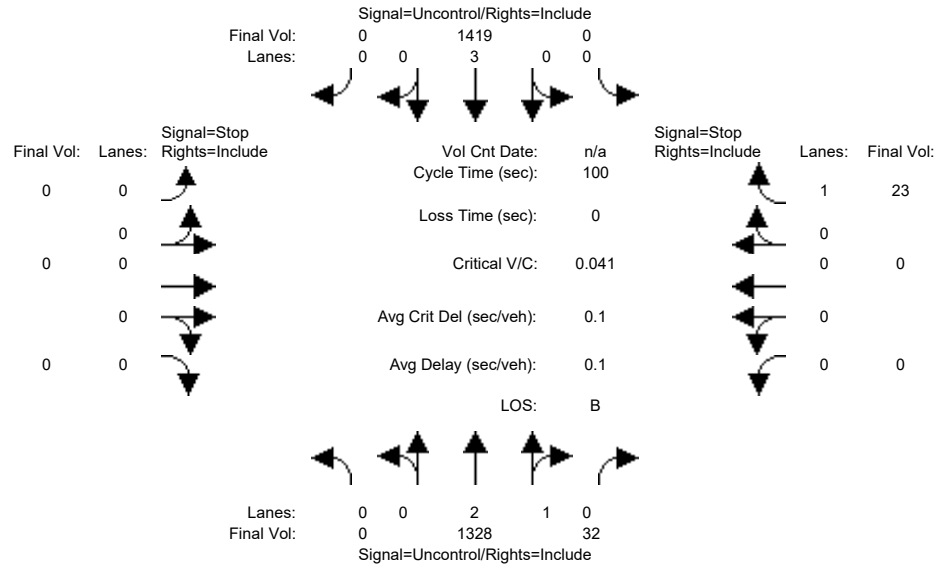


Table with columns for Street Name, Approach, and Movement, detailing volume modules for Saratoga Avenue and Mitzi Drive.

Table for Critical Gap Module showing Critical Gap, FollowUpTime, and other metrics.

Table for Capacity Module showing Conflict Vol, Potent Cap, Move Cap, and Volume/Cap.

Table for Level Of Service Module showing 2Way95thQ, Control Del, LOS by Move, and various performance metrics.

Table for Pedestrian and Vehicular metrics including HevVeh, Grade, Peds/Hour, and Pedestrian Walk Speed.

Upstream Signals: Link Index, Dist(miles), Speed (mph), SignalIndex, Cycle Time, InitVolume, Saturation, ArrivalType, G/C, and various computation results for queue clearing and platoon blocking.

UpstreamAdj:1.00 x.xxx x.xxx 1.00 x.xxx x.xxx 1.00 1.000 1.000 1.00 1.000 1.000
 Potent Cap.:1636 xxxxxx xxxxxx 1636 xxxxxx xxxxxx 46 19 543 71 20 555
 Peak Hour Delay Signal Warrant Report

 Intersection #3 Saratoga Avenue/Mitzi Drive

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 0 0 2 1 0 0 0 3 0 0 0 0 0 0 0 0 0 0 0 1
 Initial Vol: 0 1328 32 0 1419 0 0 0 0 0 0 0 0 0 23
 ApproachDel: xxxxxx xxxxxx xxxxxx 11.8

 Approach[westbound][lanes=1][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.1]
 FAIL - Vehicle-hours less than 4 for one lane approach.
 Signal Warrant Rule #2: [approach volume=23]
 FAIL - Approach volume less than 100 for one lane approach.
 Signal Warrant Rule #3: [approach count=3][total volume=2802]
 SUCCEED - Total volume greater than or equal to 650 for intersection
 with less than four approaches.

 SIGNAL WARRANT DISCLAIMER
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 a traffic signal in the future. Intersections that exceed this warrant
 are probably more likely to meet one or more of the other volume based
 signal warrant (such as the 4-hour or 8-hour warrants).

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Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #3 Saratoga Avenue/Mitzi Drive

 Future Volume Alternative: Peak Hour Warrant NOT Met

 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Lanes: 0 0 2 1 0 0 0 3 0 0 0 0 0 0 0 0 0 0 0 1
 Initial Vol: 0 1328 32 0 1419 0 0 0 0 0 0 0 0 0 23

 Major Street Volume: 2779
 Minor Approach Volume: 23
 Minor Approach Volume Threshold: -67 [less than minimum of 100]

 SIGNAL WARRANT DISCLAIMER
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 a traffic signal in the future. Intersections that exceed this warrant
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 signal warrant (such as the 4-hour or 8-hour warrants).

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 the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Cumulative PM

Intersection #4: Mitzi Drive/Piper Drive

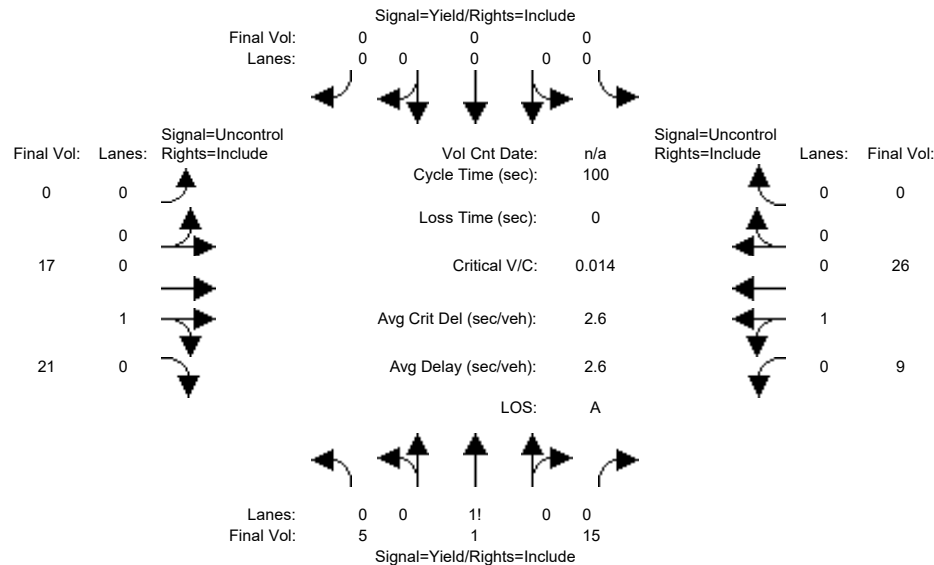


Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Peak Hour Delay Signal Warrant Report
Intersection #4 Mitzi Drive/Piper Drive
Future Volume Alternative: Peak Hour Warrant NOT Met
Signal Warrant Rule #1: [vehicle-hours=0.1]
Signal Warrant Rule #2: [approach volume=21]
Signal Warrant Rule #3: [approach count=3][total volume=94]

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future.

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction.

```

*****
Intersection #4 Mitzi Drive/Piper Drive
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R     L - T - R     L - T - R     L - T - R
-----|-----|-----|-----|-----|
Control:    Yield Sign    Yield Sign    Uncontrolled  Uncontrolled
Lanes:      0 0 1 0 0         0 0 0 0 0         0 0 0 1 0         0 1 0 0 0
Initial Vol: 5  1  15         0  0  0  0         0  17  21         9  26  0
-----|-----|-----|-----|-----|
Major Street Volume:          73
Minor Approach Volume:       21
Minor Approach Volume Threshold: 917
-----|-----|-----|-----|-----|

```

SIGNAL WARRANT DISCLAIMER
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Cumulative PM

Intersection #5: Mitzi Drive/Ranchero Way

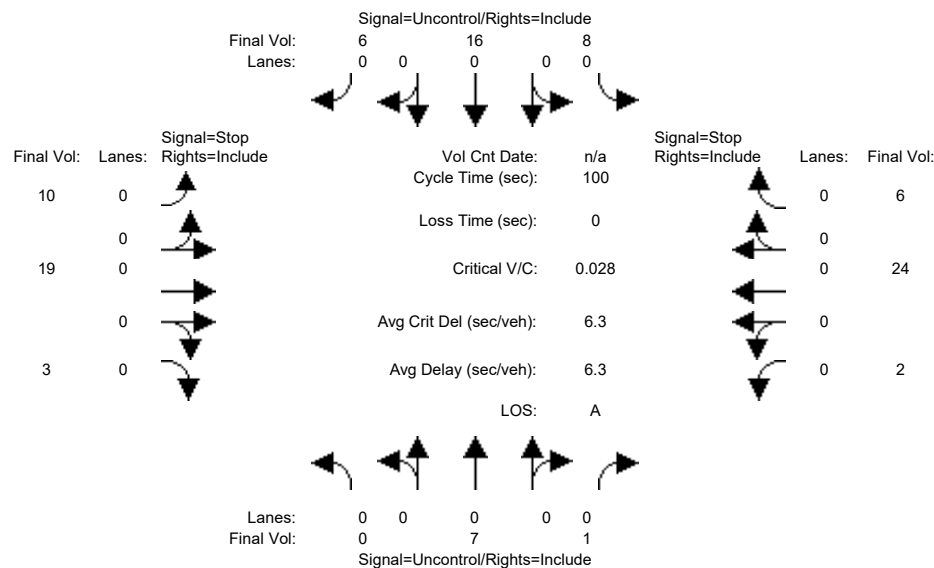


Table with columns for Street Name, Approach, and Movement. Rows include Volume Module, Critical Gap Module, Capacity Module, and Level Of Service Module.

Table with columns for Volume/Cap, LOS, and other performance metrics. Rows include Critical Gap Module, Capacity Module, and Level Of Service Module.

Note: Queue reported is the number of cars per lane.
HevVeh: 0%
Grade: 0%
Peds/Hour: 0
Pedestrian Walk Speed: 4.00 feet/sec
LaneWidth: 12 feet
Time Period: 0.25 hour

Peak Hour Delay Signal Warrant Report
Intersection #5 Mitzi Drive/Ranchero Way
Future Volume Alternative: Peak Hour Warrant NOT Met

Table with columns for Approach, Movement, Control, Lanes, Initial Vol, and ApproachDel. Rows include North Bound, South Bound, East Bound, and West Bound.

Approach[eastbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=32]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=4][total volume=102]
FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting

a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

```

*****
Intersection #5 Mitzi Drive/Ranchero Way
*****
Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R     L - T - R     L - T - R     L - T - R
-----|-----|-----|-----|-----|
Control:    Uncontrolled   Uncontrolled   Stop Sign     Stop Sign
Lanes:      0 0 0 1 0         0 0 1! 0 0     0 0 1! 0 0     0 0 1! 0 0
Initial Vol: 0 7 1         8 16 6         10 19 3         2 24 6
-----|-----|-----|-----|-----|
Major Street Volume:           38
Minor Approach Volume:         32
Minor Approach Volume Threshold: 1092
-----|-----|-----|-----|-----|

```

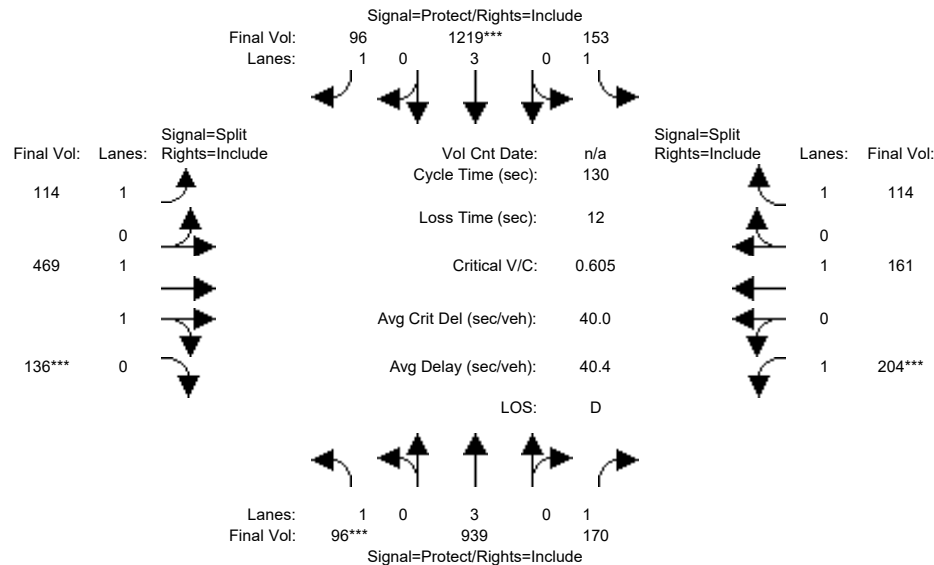
SIGNAL WARRANT DISCLAIMER

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Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative PM

Intersection #3793: Saratoga Avenue/Williams Road

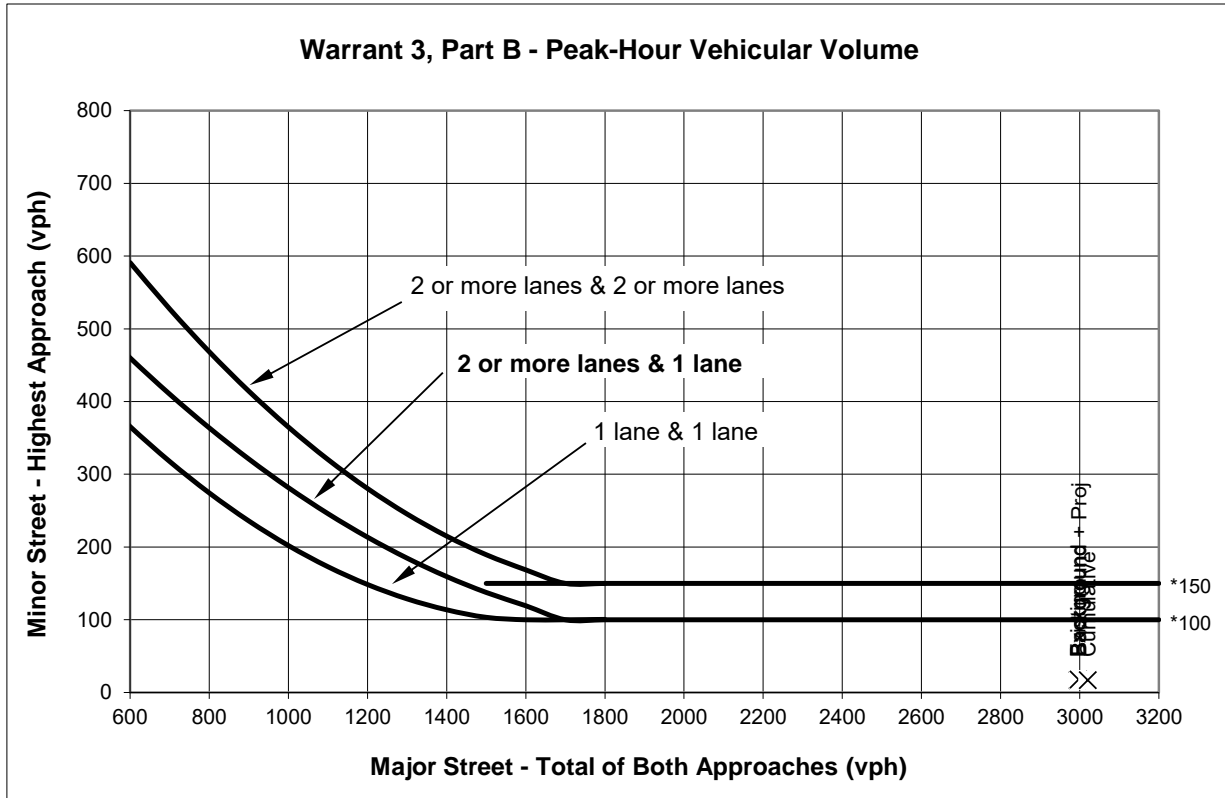


| Street Name: | Saratoga Avenue | | | | | | Williams Road | | | | | |
|---------------------------|-----------------|------|------|-------------|------|------|---------------|------|------|------------|------|------|
| | North Bound | | | South Bound | | | East Bound | | | West Bound | | |
| Movement: | L | T | R | L | T | R | L | T | R | L | T | R |
| Min. Green: | 7 | 10 | 10 | 7 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 |
| Y+R: | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Volume Module: | | | | | | | | | | | | |
| Base Vol: | 96 | 939 | 170 | 153 | 1219 | 96 | 114 | 469 | 136 | 204 | 161 | 114 |
| Growth Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Initial Bse: | 96 | 939 | 170 | 153 | 1219 | 96 | 114 | 469 | 136 | 204 | 161 | 114 |
| Added Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| PasserByVol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Initial Futr: | 96 | 939 | 170 | 153 | 1219 | 96 | 114 | 469 | 136 | 204 | 161 | 114 |
| User Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| PHF Volume: | 96 | 939 | 170 | 153 | 1219 | 96 | 114 | 469 | 136 | 204 | 161 | 114 |
| Reduct Vol: | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced Vol: | 96 | 939 | 170 | 153 | 1219 | 96 | 114 | 469 | 136 | 204 | 161 | 114 |
| PCE Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| MLF Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Final Volume: | 96 | 939 | 170 | 153 | 1219 | 96 | 114 | 469 | 136 | 204 | 161 | 114 |
| Saturation Flow Module: | | | | | | | | | | | | |
| Sat/Lane: | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Adjustment: | 0.92 | 1.00 | 0.92 | 0.92 | 1.00 | 0.92 | 0.92 | 0.98 | 0.95 | 0.92 | 1.00 | 0.92 |
| Lanes: | 1.00 | 3.00 | 1.00 | 1.00 | 3.00 | 1.00 | 1.00 | 1.54 | 0.46 | 1.00 | 1.00 | 1.00 |
| Final Sat.: | 1750 | 5700 | 1750 | 1750 | 5700 | 1750 | 1750 | 2868 | 832 | 1750 | 1900 | 1750 |
| Capacity Analysis Module: | | | | | | | | | | | | |
| Vol/Sat: | 0.05 | 0.16 | 0.10 | 0.09 | 0.21 | 0.05 | 0.07 | 0.16 | 0.16 | 0.12 | 0.08 | 0.07 |
| Crit Moves: | **** | | | **** | | | **** | | **** | **** | | |
| Green Time: | 11.8 | 37.7 | 37.7 | 20.0 | 46.0 | 46.0 | 35.2 | 35.2 | 35.2 | 25.1 | 25.1 | 25.1 |
| Volume/Cap: | 0.60 | 0.57 | 0.33 | 0.57 | 0.60 | 0.16 | 0.24 | 0.60 | 0.60 | 0.60 | 0.44 | 0.34 |
| Uniform Del: | 56.9 | 39.2 | 36.3 | 51.0 | 34.5 | 28.7 | 37.0 | 41.4 | 41.4 | 47.9 | 46.3 | 45.3 |
| IncrementDel: | 6.5 | 0.5 | 0.4 | 2.8 | 0.5 | 0.1 | 0.3 | 1.1 | 1.1 | 3.1 | 0.8 | 0.6 |
| InitQueueDel: | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Delay Adj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Delay/Veh: | 63.3 | 39.7 | 36.6 | 53.8 | 35.1 | 28.8 | 37.3 | 42.4 | 42.4 | 51.0 | 47.1 | 45.9 |
| User DelAdj: | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| AdjDel/Veh: | 63.3 | 39.7 | 36.6 | 53.8 | 35.1 | 28.8 | 37.3 | 42.4 | 42.4 | 51.0 | 47.1 | 45.9 |
| LOS by Move: | E | D | D | D | D | C | D | D | D | D | D | D |
| HCM2k95thQ: | 10 | 20 | 11 | 13 | 24 | 6 | 8 | 20 | 20 | 16 | 11 | 9 |

Note: Queue reported is the number of cars per lane.

Appendix D

Signal Warrant Sheets



Source: Figure 4C-3 California Manual on Uniform Traffic Control Devices for Streets and Highways (FHWA's MUTCD 2010 Edition, as amended for use in California).

* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Warrant 3, Part B - Peak-Hour Vehicular Volume

| | | Approach Lanes | | AM PEAK PERIOD | | | | | | | |
|--|-----------------|----------------|----------|----------------|------------|-------------------|------------|--|--|--|--|
| | | 2 or | One More | Existing | Background | Background + Proj | Cumulative | | | | |
| Major Street - Both Approaches | Saratoga Avenue | | X | 2995 | 2995 | 2997 | 3020 | | | | |
| Minor Street - Highest Approach | Piper Drive | X | | 17 | 17 | 18 | 17 | | | | |
| Signal Warranted Based on Part B - Peak-Hour Volumes? | | | | No | No | No | No | | | | |

*Warrant is satisfied if plotted points fall above the appropriate curve in graph above.

Note 1: Right turn volume was removed from the minor WB approach.

Saratoga Avenue and Piper Drive

TRAFFIC SIGNAL WARRANTS WORKSHEET

Analyst: JW date: 11/19/18

Major Street: Saratoga Avenue
 Minor Street: Piper Drive

Critical Approach Speed* (mph) 40
 Critical Approach Speed* (mph) 25
 *Posted Speed.

Critical speed of major street traffic > 50 mph (64 km/h)..... }
 In built up area of isolated community of < 10,000 population..... } **Rural (R)**
 Urban (U)

AM PEAK PERIOD

Warrant 3 - Peak Hour

The need for a traffic control signal should be considered if an engineering study finds that the criteria in either of the following two categories (Parts A and B) are met:

PART A

(All parts 1, 2, and 3 below must be satisfied)

| | AM PEAK PERIOD | | | | | | | |
|---|----------------|------------|-------------------|------------|--|--|--|--|
| | Existing | Background | Background + Proj | Cumulative | | | | |
| Minor Street Approach Direction w/ Highest Delay | WB | WB | WB | WB | | | | |
| Highest Minor Street Average Delay (sec/veh) | 93.0 | 93.0 | 89.9 | 95.8 | | | | |
| Corresponding Minor Street Approach Volume (veh/hr) | 42 | 42 | 49 | 42 | | | | |
| Minor Street Total Delay (veh-hrs) | 1.1 | 1.1 | 1.2 | 1.1 | | | | |
| Total Entering Volume (veh/hr) | 3037 | 3037 | 3046 | 3062 | | | | |
| 1. The total delay experienced for traffic on one minor street approach controlled by a STOP sign equals or exceeds 4 vehicle-hours for a 1-lane approach and 5 vehicle-hours for a 2-lane approach; <u>AND</u> | No | No | No | No | | | | |
| 2. The volume on the same minor street approach equals or exceeds 100 vph for 1 moving lane of traffic or 150 vph for 2 moving lanes; <u>AND</u> | No | No | No | No | | | | |
| 3. The total entering volume serviced during the hour equals or exceeds 800 vph for intersections with 4 or more approaches or 650 vph for intersections with 3 approaches. | Yes | Yes | Yes | Yes | | | | |
| Signal Warranted based on Part A? | No | No | No | No | | | | |

PART B

| | Approach Lanes | | AM PEAK PERIOD | | | | | | | |
|--|-----------------|-----------|----------------|------------|-------------------|------------|--|--|--|--|
| | | | Existing | Background | Background + Proj | Cumulative | | | | |
| Major Street - Both Approaches | Saratoga Avenue | 2 or More | X | | | | | | | |
| Minor Street - Highest Approach | Piper Drive | One | X | | | | | | | |
| Signal Warranted based on Part B? | | | No | No | No | No | | | | |

The Warrant is satisfied if the plotted point for vehicles per hour on the major street (both approaches) and the corresponding per hour higher vehicle volume minor street approach (one direction only) for one hour (any four consecutive 15-minute periods) fall above the applicable curves in California MUTCD Figure 4C-3 or 4C-4.

Source: California Manual on Uniform Traffic Control Devices for Streets and Highways (FHWA's MUTCD 2010 Edition, as amended for use in California)

Appendix E

San Jose Approved Trips Inventory

AM APPROVED TRIPS

10/24/2018

Intersection of: SARATOGA/WILLIAMS

Page No: 1

Traffic Node Number: 3793

| Permit No. / Description / Location | M09 NBL | M08 NBT | M07 NBR | M03 SBL | M02 SBT | M01 SBR | M12 EBL | M11 EBT | M10 EBR | M06 WBL | M05 WBT | M04 WBR |
|-------------------------------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|
| NSJ NORTH SAN JOSE | 1 | 35 | 2 | 0 | 4 | 0 | 0 | 0 | 0 | 1 | 4 | 2 |

| | | | | | | | | | | | | |
|---------------|----------|-----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|
| TOTAL: | 1 | 35 | 2 | 0 | 4 | 0 | 0 | 0 | 0 | 1 | 4 | 2 |
|---------------|----------|-----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|

| | LEFT | THRU | RIGHT |
|-------|------|------|-------|
| NORTH | 0 | 4 | 0 |
| EAST | 1 | 4 | 2 |
| SOUTH | 1 | 35 | 2 |
| WEST | 0 | 0 | 0 |

PM APPROVED TRIPS

10/24/2018

Intersection of: SARATOGA/WILLIAMS

Page No: 2

Traffic Node Number: 3793

| Permit No. / Description / Location | M09 NBL | M08 NBT | M07 NBR | M03 SBL | M02 SBT | M01 SBR | M12 EBL | M11 EBT | M10 EBR | M06 WBL | M05 WBT | M04 WBR |
|-------------------------------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|------------|
| NSJ NORTH SAN JOSE | 0 | 6 | 0 | 2 | 29 | 1 | 0 | 0 | 0 | 2 | 2 | 1 |

| | | | | | | | | | | | | |
|---------------|----------|----------|----------|----------|-----------|----------|----------|----------|----------|----------|----------|----------|
| TOTAL: | 0 | 6 | 0 | 2 | 29 | 1 | 0 | 0 | 0 | 2 | 2 | 1 |
|---------------|----------|----------|----------|----------|-----------|----------|----------|----------|----------|----------|----------|----------|

| | LEFT | THRU | RIGHT |
|-------|------|------|-------|
| NORTH | 2 | 29 | 1 |
| EAST | 2 | 2 | 1 |
| SOUTH | 0 | 6 | 0 |
| WEST | 0 | 0 | 0 |